

ADELAIDE HILLS COUNCIL

AMY GILLETT BIKEWAY EXTENSION

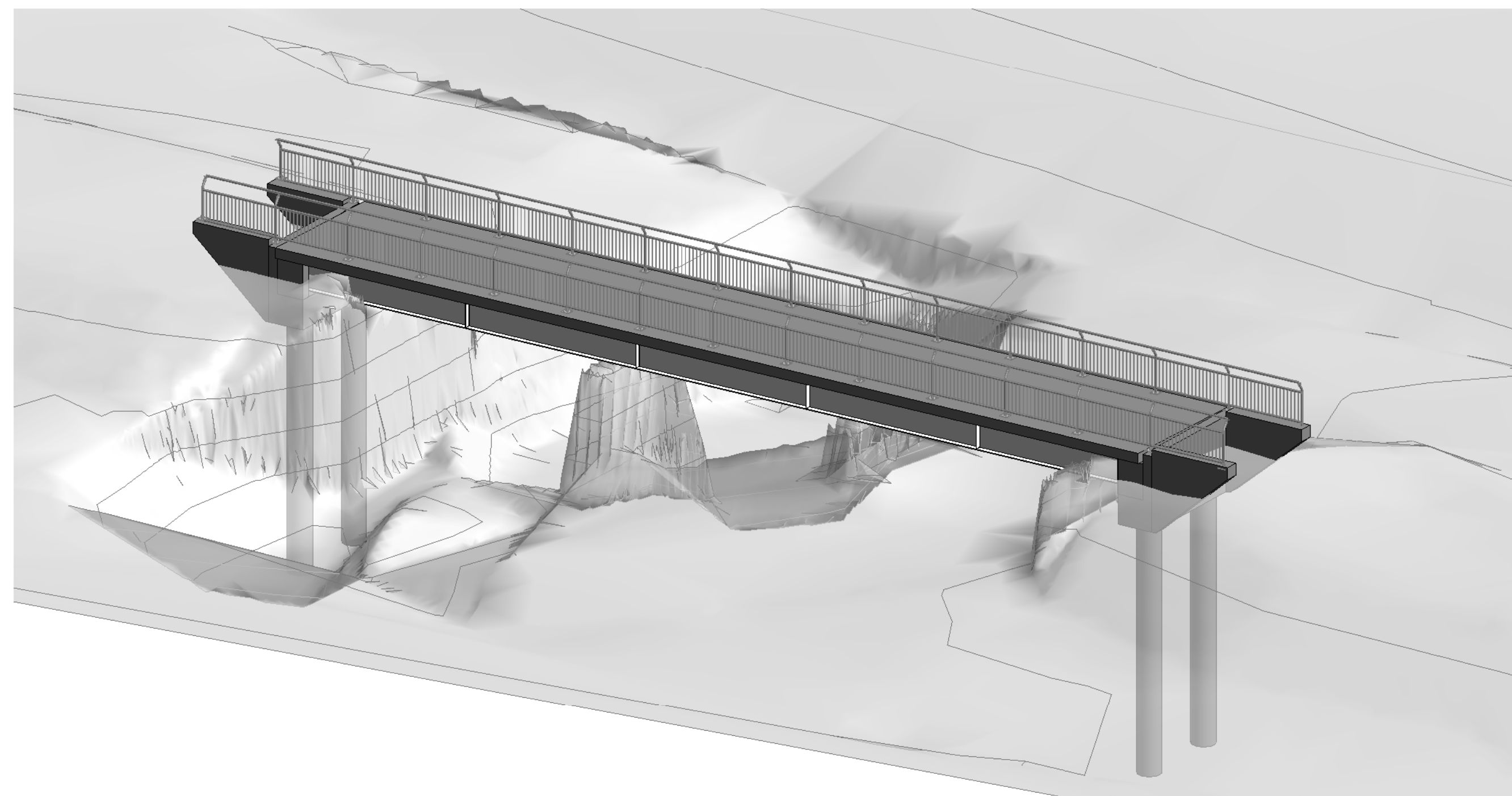
ANGAS CREEK AND WILLIAMS CREEK

NEW BRIDGES

12638597



LOCATION PLAN



ORTHOGRAPHIC PROJECTION DEPICTING ANGUS CREEK
FOOTBRIDGE

DRAWING INDEX	
DRAWING NUMBER	DRAWING TITLE
1263897-GHD-00-00-DRG-ST-00001	COVER SHEET & DRAWING INDEX
1263897-GHD-00-00-DRG-ST-00010	STRUCTURAL NOTES
1263897-GHD-00-00-DRG-ST-00101	ANGAS CREEK BRIDGE GENERAL ARRANGEMENT
1263897-GHD-00-00-DRG-ST-00102	ANGAS CREEK BRIDGE PILE SETOUT PLAN
1263897-GHD-00-00-DRG-ST-00103	ANGAS CREEK BRIDGE BEARING SETOUT PLAN
1263897-GHD-00-00-DRG-ST-00104	ANGAS CREEK BRIDGE WING WALLS LAYOUT PLAN & SECTION
1263897-GHD-00-00-DRG-ST-00105	ANGAS CREEK BRIDGE SECTIONS & DETAILS
1263897-GHD-00-00-DRG-ST-00201	WILLIAMS CREEK BRIDGE GENERAL ARRANGEMENT
1263897-GHD-00-00-DRG-ST-00202	WILLIAMS CREEK BRIDGE PILE SETOUT PLAN
1263897-GHD-00-00-DRG-ST-00203	WILLIAMS CREEK BRIDGE BEARING SETOUT PLAN
1263897-GHD-00-00-DRG-ST-00204	WILLIAMS CREEK BRIDGE WING WALLS LAYOUT PLAN & SECTION
1263897-GHD-00-00-DRG-ST-00205	WILLIAMS CREEK BRIDGE SECTIONS & DETAILS
1263897-GHD-00-00-DRG-ST-00301	TYPICAL PILE DETAILS
1263897-GHD-00-00-DRG-ST-00305	TYPICAL ABUTMENT CONCRETE DETAILS
1263897-GHD-00-00-DRG-ST-00306	TYPICAL ABUTMENT REINFORCEMENT DETAILS
1263897-GHD-00-00-DRG-ST-00311	TYPICAL WING WALLS DETAILS
1263897-GHD-00-00-DRG-ST-00315	TYPICAL STEEL GIRDER DETAILS - SHEET 1
1263897-GHD-00-00-DRG-ST-00316	TYPICAL STEEL GIRDER DETAILS - SHEET 2
1263897-GHD-00-00-DRG-ST-00321	TYPICAL BEARING DETAILS
1263897-GHD-00-00-DRG-ST-00325	TYPICAL EXPANSION JOINT DETAILS
1263897-GHD-00-00-DRG-ST-00331	TYPICAL PRECAST DECK PANEL LAYOUT
1263897-GHD-00-00-DRG-ST-00332	TYPICAL PRECAST DECK PANEL DETAILS
1263897-GHD-00-00-DRG-ST-00335	TYPICAL IN SITU DECK REINFORCEMENT
1263897-GHD-00-00-DRG-ST-00401	BURFORDS HILL ROAD BRIDGE - REMEDIAL WORKS PLAN AND ELEVATION
1263897-GHD-00-00-DRG-ST-00402	BURFORDS HILL ROAD BRIDGE - DECK RETROFIT DETAILS

NOT FOR CONSTRUCTION

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Rev	Description	Checked	Approved	Date
Author	A. PERNA	Drafting Check	L. PARRY	
Designer	C. NGUYEN	Design Check	A. RITCHIE	

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Project No.
12638597

Client	ADELAIDE HILLS COUNCIL
Project	AMY GILLETT BIKEWAY EXTENSION
Status	PRELIMINARY

Drawing Title COVER SHEET & DRAWING INDEX

Drawing No.
12638597-GHD-00-00-DRG-ST-00001

Size
A1

Rev
P01

GENERAL:

1. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING THE STRUCTURE IN A STABLE CONDITION AND ENSURING NO PART SHALL BE OVER STRESSED DURING CONSTRUCTION ACTIVITIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF ALL TEMPORARY WORKS.
2. WORKMANSHIP AND MATERIAL SHALL BE IN ACCORDANCE WITH THE DRAWINGS AND DIT MASTER SPECIFICATION FOR TRANSPORT INFRASTRUCTURE (DIT MASTER SPECIFICATION). WHERE NOT SPECIFIED THEY SHALL BE IN ACCORDANCE WITH RELEVANT AUSTRALIAN STANDARDS.
3. NOMINATION OF PROPRIETARY ITEMS DOES NOT INDICATE EXCLUSIVE PREFERENCE BUT INDICATES REQUIRED PROPERTIES OF THE ITEM. SIMILAR ALTERNATIVE PRODUCTS HAVING EQUIVALENT FUNCTIONS OR PERFORMANCE MAY BE OFFERED FOR APPROVAL. PROPRIETARY ITEMS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S REQUIREMENTS.
4. LEGEND:

HDG - DENOTES HOT DIP GALVANISED

CJ - DENOTES CONSTRUCTION JOINT

UNO = UNLESS NOTED OTHERWISE

ENGINEER = NOMINATED REPRESENTATIVE OF GHD PTY LTD

SLS = SERVICEABILITY LIMIT STATE

ULS = ULTIMATE LIMIT STATE

NS = NATURAL SURFACE LEVEL

NA = NOT APPLICABLE

FSL = FINISHED SURFACE LEVEL

DSL = DESIGN SURFACE LEVEL

ESL = EXISTING SURFACE LEVEL
5. ALL RLs NOTED ARE IN AHD.
6. DIMENSIONS SHALL NOT BE SCALED FROM THE DRAWINGS. ALL DIMENSIONS RELEVANT TO SETTING OUT AND OFF-SITE FABRICATION SHALL BE CONFIRMED AND VERIFIED BY THE CONTRACTOR PRIOR TO COMMENCING CONSTRUCTION. THE CONTRACTOR SHALL REPORT ALL DISCREPANCIES TO THE DESIGNER.
7. BUILD, FABRICATE AND PROCURE ONLY FROM DRAWINGS 'ISSUED FOR CONSTRUCTION'.

DESIGN SPECIFICATION:

1. DESIGN STANDARD: AS5100:2017 BRIDGE DESIGN INCLUDING ALL AMENDMENTS AND DIT MASTER SPECIFICATION ST-SD-D1 'DESIGN OF STRUCTURES'.
2. DESIGN LIFE - 100 YEARS
3. (i) PEDESTRIAN AND CYCLIST PATH LOAD: 5.0kPa (UNIFORM), 20kN (CONCENTRATED)

(ii) EARTHQUAKE:

HAZARD FACTOR (Z)

PROBABILITY FACTOR (kp)

SITE SUB-SOIL CLASS

STRUCTURAL DUCTILITY FACTOR (μ)

BRIDGE EARTHQUAKE DESIGN CATEGORY

0.1

1.3

Ca

2.0 (DAMAGE CONTROL),1.0 (ABUTMENTS)

BEDC - 2

(iii) WIND LOAD:

REGIONAL WIND SPEED

48m/s (ULS)

37m/s (SLS)

REGION : A1

(iv) THERMAL EFFECTS

AVERAGE BRIDGE TEMPERATURE CHANGE

+41°C / -20°C

(v) CONSTRUCTION LIVE LOAD

(vi) FLOOD LOAD:
- | | ANGAS CREEK | | WILLIAMS CREEK | |
|-------------|---------------|----------------|----------------|----------------|
| EVENT (ARI) | LEVEL (m AHD) | VELOCITY (m/s) | LEVEL (m AHD) | VELOCITY (m/s) |
| 100 YEARS | 378.70 | 5.08 | 380.00 | 3.32 |
| 2000 YEARS | 379.61 | 6.79 | 380.93 | 4.34 |
- DUE TO POSITION OF WATER LEVEL RELATIVE TO GIRDER SOFFIT, WATER LOADING HAS NOT BEEN CONSIDERED.

(vi) MINIMUM LATERAL RESTRAINT LOAD200 kN.

4. THE DESIGN IS BASED UPON THE FOLLOWING BACKFILL PARAMETERS BEHIND THE ABUTMENTS AND WING WALLS.

MATERIAL DENSITY = 20 kN/m3

CHARACTERISTIC FRICTION ANGLE = 30 deg
- CONCRETE:
1. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH THE SPECIFICATION AND AS5100:2017 BRIDGE DESIGN.

2. EXPOSURE CLASSIFICATION "A" FOR INTERFACE BETWEEN INSITU AND PRECAST DECK PANELS AND "B1" FOR ALL OTHER CONCRETE ELEMENTS.

3. CONCRETE TO BE USED IN EACH ELEMENTS OF THE WORK SHALL BE OF THE GRADE SHOWN BELOW UNLESS STATED OTHERWISE ON THE DRAWINGS. THE GRADE DESIGNATION SPECIFIES THE CONCRETE STRENGTH. THE NOMINAL AGGREGATE SIZE SHALL BE 20mm, HOWEVER WHERE COVER IS SPECIFIED AS 25mm THE MAXIMUM AGGREGATE SIZE SHALL NOT BE MORE THAN 14mm
- | STRUCTURAL ELEMENT | CONCRETE GRADE | CONCRETE COVER (mm) | | | PRECAST # | |
|---|----------------|---------------------|----------|--------|-----------|--|
| | | CAST AGAINST | | | | |
| | | FORMS | BLINDING | GROUND | | |
| PILE CAPPING PILE | S40 | 45 | 55 | 75 | N/A | |
| ABUTMENTS / WING WALLS | S40 | 45 | 55 | 75 | N/A | |
| PILE PLUG | S40 | 50 | N/A | N/A | N/A | |
| PRECAST PANEL DECK | S50 | N/A | N/A | N/A | 30 | |
| INSITU DECK SLAB - TOP / SIDES | S40 | 45 | N/A | N/A | N/A | |
| INSITU DECK SLAB - BOTTOM (CAST ON PERMANET FORMWORK) | S40 | 30 | N/A | N/A | N/A | |
| BLINDING CONCRETE | N15 | N/A | N/A | N/A | N/A | |
- * INCREASE COVER BY 5mm WHERE CURING COMPOUNDS ARE USED. CURING COMPOUNDS SHALL NOT BE USED UNLESS APPROVED BY THE DESIGNER.

90mm IF CAST AGAINST GROUND

RIGID FORMWORK AND INTENSE COMPACTION TO BE USED

4. UNLESS SPECIFICALLY NOTED OTHERWISE ON THE DRAWINGS, ALL EXPOSED CONCRETE EDGES HAVING A CONTAINED ANGLE LESS THAN 120° SHALL BE PROVIDED WITH 20mm FILLETS OR CHAMFERS AS APPROPRIATE.

5. CONSTRUCTION JOINTS SHALL BE USED ONLY AS SHOWN ON THE DRAWINGS.

6. NO CONSTRUCTION JOINTS SHOWN ON THE DRAWINGS SHALL BE OMITTED WITHOUT WRITTEN APPROVAL OF THE DESIGNERS.
- LIFTING AND HANDLING OF PRECAST ELEMENTS:
1. OILED SMOOTH STEEL FORMS OR APPROVED EQUIVALENT TO BE USED DURING CASTING.

2. PRECAST ELEMENTS SHALL ONLY BE LIFTED USING INSTALLED LIFTING ANCHORS.

3. LIFTING ANCHOR SIZE AND CAPACITY SHALL BE CONFIRMED BY THE PRECAST FABRICATOR AND SUBMITTED TO THE TEMPORARY WORKS DESIGN ENGINEER FOR APPROVAL, UNLESS OTHERWISE NOTED IN THE DRAWINGS.
- PRECAST CONCRETE:
1. ALL PRECAST CONCRETE SHALL COMPLY WITH THE CONCRETE NOTES ON THESE DRAWINGS AS 5100.5:2017 AND DIT MASTER SPECIFICATION ST-SC-S3.

2. THE CONSTRUCTOR SHALL REFER TO RELEVANT DRAWINGS TO ENSURE THAT ALL OPENINGS, RECESSES, FIXINGS AND FITTINGS SPECIFIED ON THE DRAWINGS ARE INCORPORATED INTO PRECAST ELEMENTS.

3. ALL FERRULES USED SHALL BE GALVANISED STEEL OR OTHERWISE APPROVED, AND FITTED WITH ANCHORAGE BARS OF MINIMUM 10mm DIAMETER. THE FERRULES SHALL DEVELOP THE FULL CAPACITY OF THE BOLT TO BE USED IN CONJUNCTION WITH THE FERRULE. A MINIMUM M20 DIAMETER FERRULE IS TO BE ADOPTED UNLESS NOTED OTHERWISE.

4. FERRULES THAT WILL BE EXPOSED AFTER COMPLETION OF ERECTION ARE TO BE RECESSED 30mm BELOW THE CONCRETE SURFACE AND ARE TO BE GROUTED ON COMPLETION WITH NON SHRINK GROUT. BONDING AGENT TO BE APPLIED TO SURFACE PRIOR TO GROUTING.

5. ALL HOT DIP GALVANISED FERRULES AND FIXING SHALL BE IN ACCORDANCE WITH AS/NZS 4680. MINIMUM COATING THICKNESS IS 85 MICRONS.

6. GROUT TO BE USED SHALL BE NON-SHRINK, AND SHALL HAVE 28 DAYS CHARACTERISTIC CYLINDER STRENGTH OF 50MPa. DETAILS OF THE PROPOSED GROUT TO BE SUBMITTED TO THE ENGINEER FOR APPROVAL.
- STRUCTURAL STEELWORK:
1. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS5100.6:2017.

2. ROLLED STEEL SECTIONS SHALL BE GRADE 300 COMPLYING WITH AS/NZS 3679 UNLESS NOTED OTHERWISE.

3. STEEL PLATES SHALL BE GRADE 350 AND COMPLY WITH AS/NZS 3678

4. BOLTS AND NUTS TO AS/NZS 1252 GRADE 8.8/TF UNLESS NOTED OTHERWISE.

5. ALL BOLTS, NUTS, WASHERS AND COUPLERS SHALL BE HOT DIP GALVANISED

6. ALL BOLTED CONNECTIONS SHALL HAVE 10 THICK PLATE AND 2-M20 8.8/TF BOLTS MIN. U.N.O

7. WELDING ELECTRODES SHALL BE E49XX UNLESS NOTED OTHERWISE AND WELD CATEGORY SP AND COMPLY WITH AS/NZ 1554 - PART 1, U.N.O.

8. ALL WELDS TO BE 8mm CONTINUOUS FILLET WELDS UNLESS NOTED OTHERWISE.

9. WELDING SYMBOLS ARE TO AS 1101.3

10. STRUCTURAL STEELWORK SHALL HAVE PROTECTIVE COATING IN ACCORDANCE WITH DIT MASTER SPECIFICATION ST-SD-D1.

11. WHERE GALVANISING IS INDICATED ON THE DRAWINGS, GALVANISING SHALL COMPLY WITH AS/NZS 4680. ALL DAMAGE TO THE GALVANISING SHALL BE REPAIRED BY THE APPLICATION OF TWO COATS OF ORGANIC ZINC RICH PRIMER AS APPROVED UNDER APAS-C-29/16, TO A DRY FILM THICKNESS OF 100 MICRONS.

12. ALL SHARP EDGES AND BURRS RESULTING FROM PUNCHING, CUTTING AND DRILLING SHALL BE REMOVED PRIOR TO GALVANISING.

13. SHEAR STUDS SHALL BE FUSION WELDED TO AS/NZS 1554.2.

14. IF THE REMOVAL OF UNACCEPTABLE SHEAR STUDS REQUIRES REPAIR TO STEEL COMPONENT FROM WHICH THE STUDS WAS REMOVED, THEN THIS SHALL BE UNDERTAKEN TO AS/NZS 1554.1 (GENERALLY).

15. UNLESS NOTED OTHERWISE ALL SHEAR STUDS SHALL BE 22mm DIAMETER.

16. OVERSPRAY OF STEELWORK PROTECTIVE COATING TO SHEAR STUDS IS PERMITTED.
- REINFORCEMENT:
1. REINFORCEMENT SYMBOLS:

N - HOT ROLLED DEFORMED BARS TO AS/NZS 4671 GRADE 500N

R - STRUCTURAL GRADE PLAIN ROUND BAR AS/NZS 4671 GRADE 250

2. REINFORCING MESH TO BE GRADE D500L TO AS/NZS 4671

3. LAP LENGTHS AND OTHER SPLICES IN REINFORCEMENT SHALL ONLY BE MADE AT THE POSITION SHOWN ON THE DRAWINGS, UNLESS ALTERNATIVE OR EXTRA LOCATIONS ARE GIVEN IN WRITING BY THE ENGINEER. LAP LENGTHS SHALL BE AS TABULATED BELOW UNLESS SHOWN OTHERWISE ON DRAWINGS.
- | MINIMUM DEVELOPMENT AND SPlicing LENGTHS (UNLESS NOTED OTHERWISE) | | | | | | | |
|---|-----|-----|-----|------|------|------|------|
| BAR SIZE | N12 | N16 | N20 | N24 | N28 | N32 | N36 |
| MINIMUM LAP LENGTH | 400 | 550 | 750 | 1000 | 1300 | 1600 | 1900 |
- NOTE: THE MINIMUM LAP LENGTH SHOWN SHALL BE INCREASED BY 30% FOR HORIZONTAL BARS WITH 300mm OR MORE CONCRETE CAST BELOW THE BAR.
- NO MORE THAN 50% OF THE BARS SHALL BE LAPPED AT ANY LOCATION WHERE STAGGERED BAR SPLICES ARE NOT POSSIBLE. THE MINIMUM LAP LENGTH SHALL NOT BE LESS THAN 1.25 TIMES THE STANDARD LAP LENGTH.
- THE MINIMUM LENGTH OF A LAPPED SPLICE FOR DEFORMED BARS IN COMPRESSION SHALL BE 40 TIMES THE BAR DIAMETER.
4. REINFORCEMENT SPACING NOT SHOWN SHOULD BE TAKEN AS EQUAL.

5. REINFORCING BARS SHOWN ON THESE DRAWINGS ARE DIAGRAMMATIC ONLY.

6. BARS SHOWN MAY REPRESENT MORE THAN ONE LENGTH AND/ OR PROFILE.

7. BARS MAY NOT BE SHOWN IN TRUE POSITION FOR CLARITY.

8. ALL HOOKS, BENDS AND COGS ARE STANDARD AND SHALL BE IN ACCORDANCE WITH AS5100.5 UNLESS NOTED OTHERWISE. THE MINIMUM LENGTH OF A LAPPED SPLICE FOR DEFORMING BARS IN COMPRESSION SHALL BE 40 TIMES THE BAR DIAMETER.

9. SITE BENDING OF REINFORCEMENT BARS SHALL BE DONE WITHOUT HEATING USING MECHANICAL BENDING TOOLS.

10. DO NOT WELD OR HEAT REINFORCEMENT UNLESS SHOWN ON DRAWINGS OR OTHERWISE APPROVED BY NOMINATED AUTHORITY.WHERE WELDING OF REINFORCEMENT IS ACCEPTED BY THE ENGINEER, THE WELDING OF REINFORCEMENT SHALL BE IN ACCORDANCE WITH AS1554.3. THE SIZE AND LENGTH OF SINGLE SIDE SP WELD USING E49XX ELECTRODES SHALL BE AS TABULATED BELOW UNLESS OTHERWISE SHOWN ON THE DRAWINGS.
- | MINIMUM WELD SIZE AND LENGTHS (UNLESS NOTED OTHERWISE) | | | | | | |
|--|-----|-----|-----|-----|-----|-----|
| BAR SIZE | N12 | N20 | N24 | N28 | N32 | N36 |
| WELD SIZE (mm) | 6 | 8 | 10 | 12 | 12 | 14 |
| WELD LENGTH (mm) | 160 | 200 | 240 | 280 | 320 | 360 |
- WELDING OF REINFORCING BARS SHALL NOT BE MADE WITHIN 3db FROM THAT PART OF A BAR THAT HAS BEEN BENT AND RE-STRAIGHTENED.
11. ALL BARS CROSSING THE CONSTRUCTION JOINTS SHALL BE HOT DIP GALVANISED UNLESS NOTED OTHERWISE.
- DEMOLITION, DISMANTLING AND CONSTRUCTION
1. DURING CONSTRUCTION, THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING THE STRUCTURE. TEMPORARY BRACING, PROPPING OR OTHER TEMPORARY WORKS SHALL BE DESIGNED AND PROVIDED BY THE CONTRACTOR AS REQUIRED

2. THE CONTRACTOR SHALL SUBMIT A METHOD STATEMENT FOR DEMOLITION AND DISMANTLING WORKS FOR APPROVAL PRIOR TO THE COMMENCEMENT OF CONSTRUCTION

3. ALL DISMANTLED MATERIALS SHALL BE STORED IN A DESIGNATED SECURE AREA BEFORE BEING DISPOSED OF OFF-SITE

4. CARE SHALL BE TAKEN TO ENSURE THAT NO DEMOLITION MATERIALS ENTER THE WATERWAY AREA

5. EXISTING SERVICES ON THE BRIDGE AND WITHIN THE CONSTRUCTION ZONE SHALL BE IDENTIFIED AND RELOCATED PRIOR TO ALL OTHER WORK. THE CONTRACTOR SHALL GAIN PERMISSION FROM THE RELEVANT AUTHORITY BEFORE TEMPORARILY OR PERMANENTLY RELOCATING ANY EXISTING SERVICES.

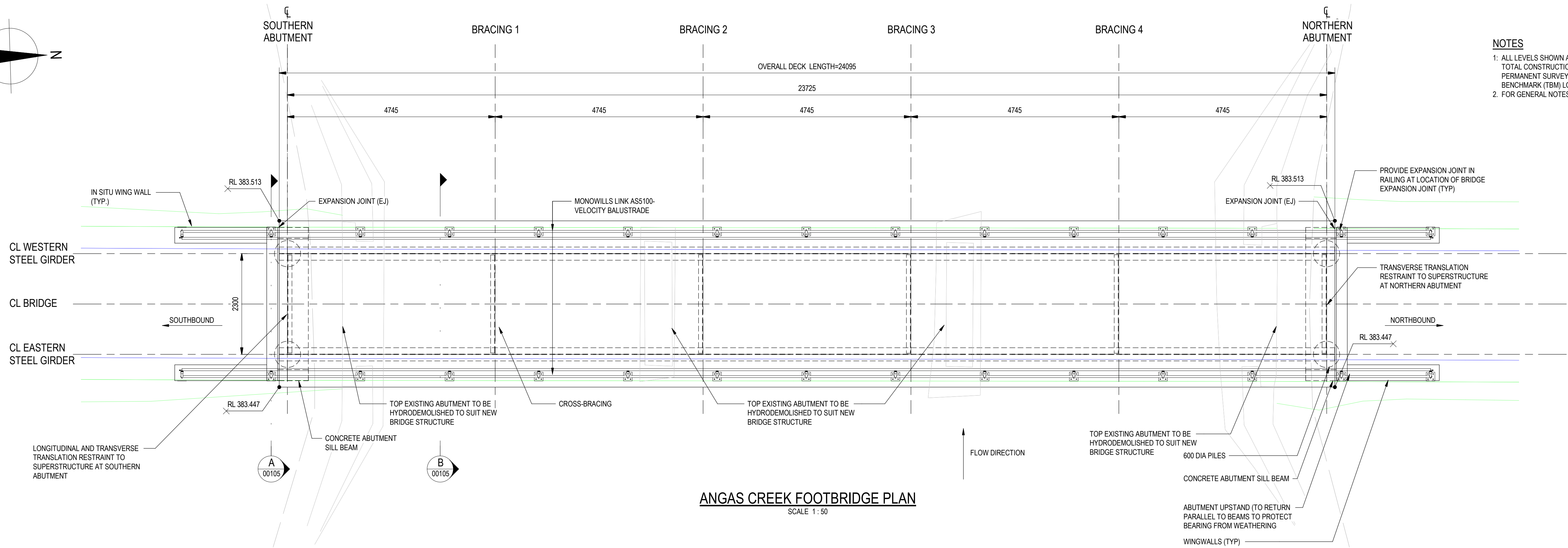
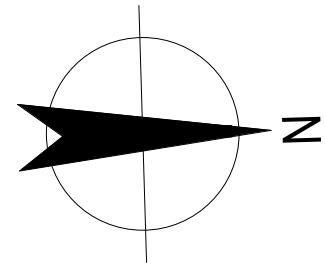
6. EXISTING BRIDGE CONCRETE ABUTMENTS AND PIERS TO BE DEMOLISH TO LEVEL INDICATED ON DRAWINGS. EXPOSED REINFORCEMENT IN THE EXISTING ABUTMENT OR PIERS AFTER CONCRETE DEMOLITION SHALL HAVE 25mm GROUND OFF INTO THE CUT SURFACE. A LAYER OF FOSROC CONBEXTRA EP65 PLUS EPOXY GROUT OR APPROVED EQUIVALENT SHALL BE APPLIED.
- REFERENCE DOCUMENTS
1. THE FOLLOWING DOCUMENTS SHALL BE REFERRED TO TO SUPLEMENT THE DESIGN DRAWINGS:

- GHD STRUCTURAL DESIGN BASIS REPORT 12638597-00000-REP-001

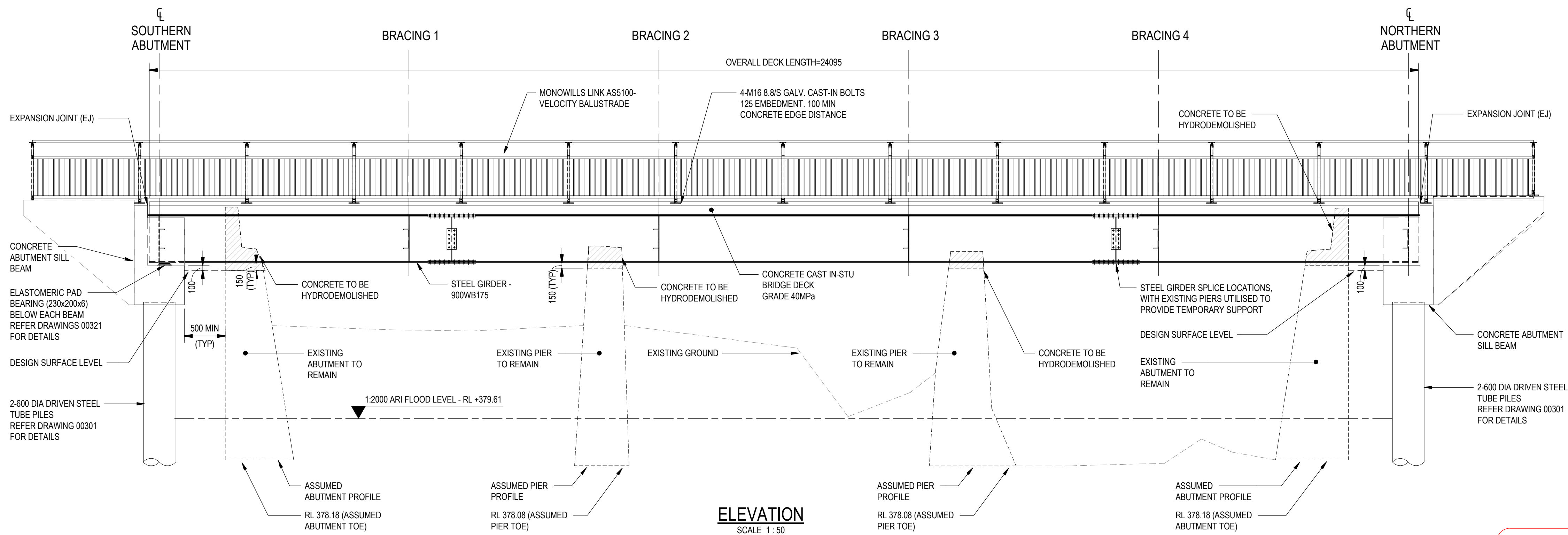
- GHD STRUCTURAL DETAILED DESIGN REPORT 12638597-00000-REP-002

- JACOBS GEOTECHNICAL INVESTIGATION REPORT IW278200-CG-RPT-002 DATED 26 JULY 2022

- DIT MASTER SPECIFICATIONS, AVAILABLE THROUGH THE DIT WEBSITE
- | | | | | | |
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|---------|------------------------------|
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| Project | AMY GILLET BIKEWAY EXTENSION |
| Status | PRELIMINARY |
- | | |
|---------------|---------------------------------|
| Drawing Title | STRUCTURAL NOTES |
| Drawing No. | 12638597-GHD-00-00-DRG-ST-00010 |
| Rev | P01 |
- NOT FOR CONSTRUCTION

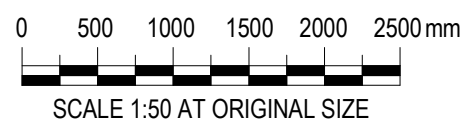


- NOTES**
1. ALL LEVELS SHOWN ARE AHD. REFER SURVEY BY TOTAL CONSTRUCTION SURVEY PTY LTD FOR PERMANENT SURVEY MARK (PSM) AND TEMPORARY BENCHMARK (TBM) LOCATIONS.
 2. FOR GENERAL NOTES REFER TO DRG 00010.



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Designer	C. NGUYEN	Design Check	A. RITCHIE	



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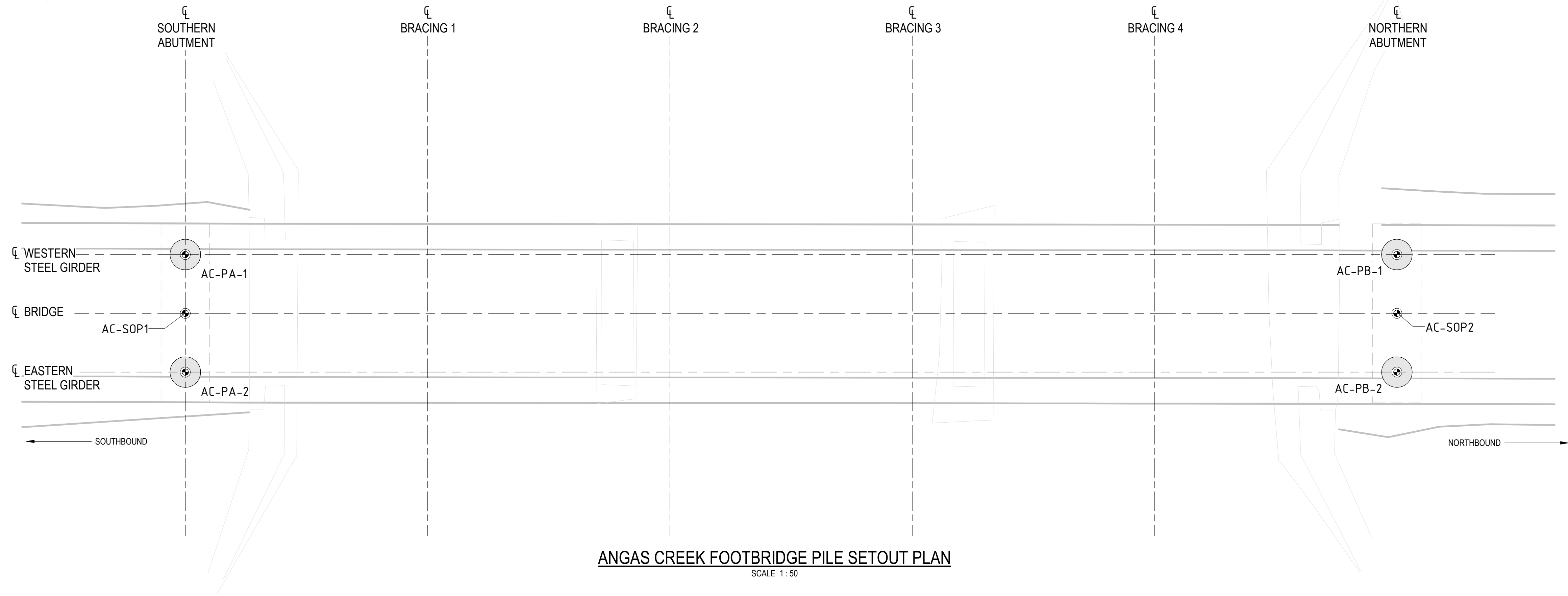
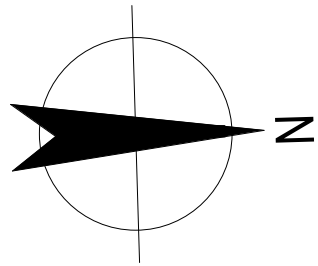
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Project **AMY GILLETT BIKEWAY EXTENSION**
Status **PRELIMINARY**

Drawing Title **ANGAS CREEK BRIDGE GENERAL ARRANGEMENT**

12638597-GHD-00-00-DRG-ST-00101

Size
A1

Rev
P01



NOTES

1. ALL LEVELS SHOWN ARE AHD. REFER SURVEY BY TOTAL CONSTRUCTION SURVEY PTY LTD FOR PERMANENT SURVEY MARK (PSM) AND TEMPORARY BENCHMARK (TBM) LOCATIONS.
2. FOR GENERAL NOTES REFER TO DRG 00010.

PILE NOTES:

1. ESTIMATED PILE TOE LEVEL CORRESPONDS TO THE PILE DESIGN GEOTECHNICAL STRENGTH ($R_{d,g}$) BEING EQUAL TO OR GREATER THAN THE DESIGN ACTION EFFECT (E_d) ie: $R_{d,g} \geq E_d$
2. THE PILE DESIGN GEOTECHNICAL STRENGTH ($R_{d,g}$) SHALL BE CALCULATED AS THE DESIGN ULTIMATE GEOTECHNICAL STRENGTH ($R_{u,g}$) MULTIPLIED BY THE GEOTECHNICAL STRENGTH FACTOR (ϕ_g) ACCORDING TO THE FOLLOWING EQUATION $R_{d,g} = \phi_g R_{u,g}$
3. THE GEOTECHNICAL STRENGTH REDUCTION FACTOR (ϕ_g) ADOPTED FOR DESIGN IS 0.7 IN ACCORDANCE WITH AS2159 FOR AXIAL COMPRESSION.
4. ONE PILE FROM EACH PILE GROUP HAS BEEN SELECTED (REFER SHEET No. 4111) FOR DYNAMIC LOAD TESTING WITH SIGNAL MATCHING ANALYSIS. THE TEST SHALL BE PERFORMED TO VERIFY THE DESIGN ULTIMATE GEOTECHNICAL STRENGTH ($R_{u,g}$) OF THE PILE.
5. THE GEOTECHNICAL DESIGN ASSUMES A GROUND WATER LEVEL OF 379.61m AUSTRALIAN HEIGHT DATUM. THE GROUNDWATER IS ASSUMED TO COINCIDE WITH THE MODELLED 0.05% AEP FLOOD LEVEL AT EACH BRIDGE. VARIATION IN GROUNDWATER LEVEL IS EXPECTED AND PERCHED WATER MAY BE PRESENT IN THE UPPER SOIL PROFILE. THE CONTRACTOR SHALL RECORD GROUNDWATER INFORMATION INCLUDING DEPTH AND ELEVATION AND PROVIDE THE RECORDED GROUNDWATER INFORMATION TO THE DESIGNER.

BRIDGE PILE SCHEDULE - SOUTHERN...				
MARK	DIAMETER (mm)	EASTING (m)	NORTHING (m)	MINIMUM LENGTH OF PILE (mm)
AC-PA-1	600	313454.699	6143770.019	10000
AC-PA-2	600	313456.998	6143769.950	10000

BRIDGE SETOUT TABLE		
SOP MARK	EASTING (m)	NORTHING (m)
AC-SOP1	313455.848	6143769.984
AC-SOP2	313456.559	6143793.689

NOTE:
BRIDGE SET-OUT POINTS (SOP) PROVIDED AT INTERSECTION
OF BRIDGE CENTRELINE AND CENTRELINE OF ABUTMENT

BRIDGE PILE SCHEDULE - NORTHERN...				
MARK	DIAMETER (mm)	EASTING (m)	NORTHING (m)	MINIMUM LENGTH OF PILE (mm)
AC-PB-1	600	313455.409	6143793.723	10000
AC-PB-2	600	313457.708	6143793.654	10000

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P01 90% DESIGN				
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Project No.
12638597

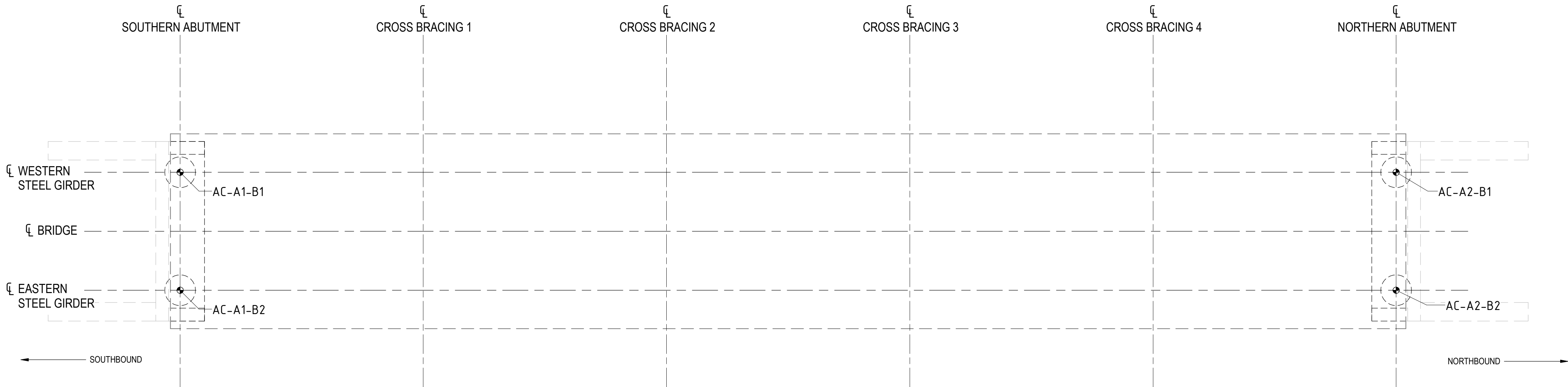
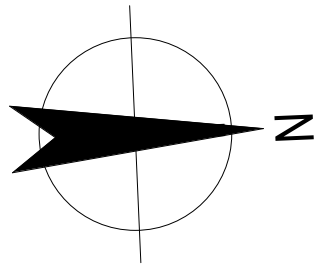
Client	ADELAIDE HILLS COUNCIL
Project	AMY GILLETT BIKEWAY EXTENSION
Status	PRELIMINARY

Drawing Title
ANGAS CREEK BRIDGE PILE SETOUT
PLAN

Drawing No.
12638597-GHD-00-00-DRG-ST-00102

Size
A1

Rev
P01



ANGAS CREEK FOOTBRIDGE BEARING SETOUT PLAN
SCALE 1:50

BEARING SET-OUT TABLE		
SOP MARK	EASTING (m)	NORTHING (m)
AC-A1-B1	313454.699	6143770.019
AC-A1-B2	313456.998	6143769.950
AC-A2-B1	313455.409	6143793.723
AC-A2-B2	313457.708	6143793.654

NOTES

1. ALL LEVELS SHOWN ARE AHD. REFER SURVEY BY TOTAL CONSTRUCTION SURVEY PTY LTD FOR PERMANENT SURVEY MARK (PSM) AND TEMPORARY BENCHMARK (TBM) LOCATIONS.
2. FOR GENERAL NOTES REFER TO DRG 00010.

BEARING NOTES:

1. ALL BEARINGS SHALL BE IN ACCORDANCE WITH AS 5100.4.
2. ALL BEARINGS SHALL BE TESTED IN ACCORDANCE WITH THE DIT MASTER SPECIFICATION ST-BF-C1.
3. ALL STEEL SHALL BE GRADE 250 TO AS/NZS 3678 UNO.
4. ALL SCREWS AND BOLTS SHALL BE HOT DIP GALVANISED.
5. STEEL ATTACHMENT PLATES AND KEEPER PLATES SHALL BE HOT DIP GALVANISED PRIOR TO ASSEMBLY.
6. TOLERANCE FOR MACHINING OF STEEL PLATES IS $\pm 0.5\text{mm}$.
7. MACHINE ALL STEEL PLATE FACES IN CONTACT WITH BEAMS.
8. ALL BEARINGS SHALL BE SET HORIZONTALLY IN ALL DIRECTIONS.
9. GROUT SHALL BE CONBEXTRA HF. STRENGTH OF GROUT PAD SHALL BE 30MPa PRIOR TO INSTALLATION OF BEARINGS. GROUT SHALL BE MIXED AND PLACED IN ACCORDANCE WITH MANUFACTURER'S SPECIFICATIONS.
10. THE SURFACE DIRECTLY BELOW THE GROUT PAD SHALL BE SCABBLED AND CLEANED TO REMOVE ALL LOOSE MATERIAL. THE AREA TO BE SCABBLED SHALL BE EQUAL TO THE BASE OF THE FINAL GROUT PAD LESS 20mm ON ALL SIDES.
11. TAPERED TOP PLATE PROVIDED TO ENSURE BEAMS LIE LEVEL UNDER PERMANENT EFFECTS.

NOT FOR CONSTRUCTION

P01	90% DESIGN	AR	LR	07.06.24
Rev	Description	Checked	Approved	Date
Author	A. PERNA	Drafting Check	L. PARRY	
Designer	C. NGUYEN	Design Check	A. RITCHIE	



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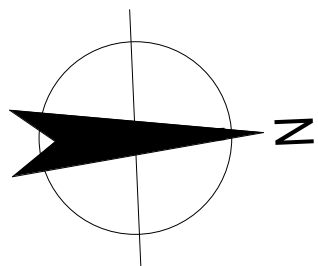
Client	ADELAIDE HILLS COUNCIL
Project	AMY GILLETT BIKEWAY EXTENSION
Status	PRELIMINARY

Drawing Title
ANGAS CREEK BRIDGE BEARING SETOUT PLAN

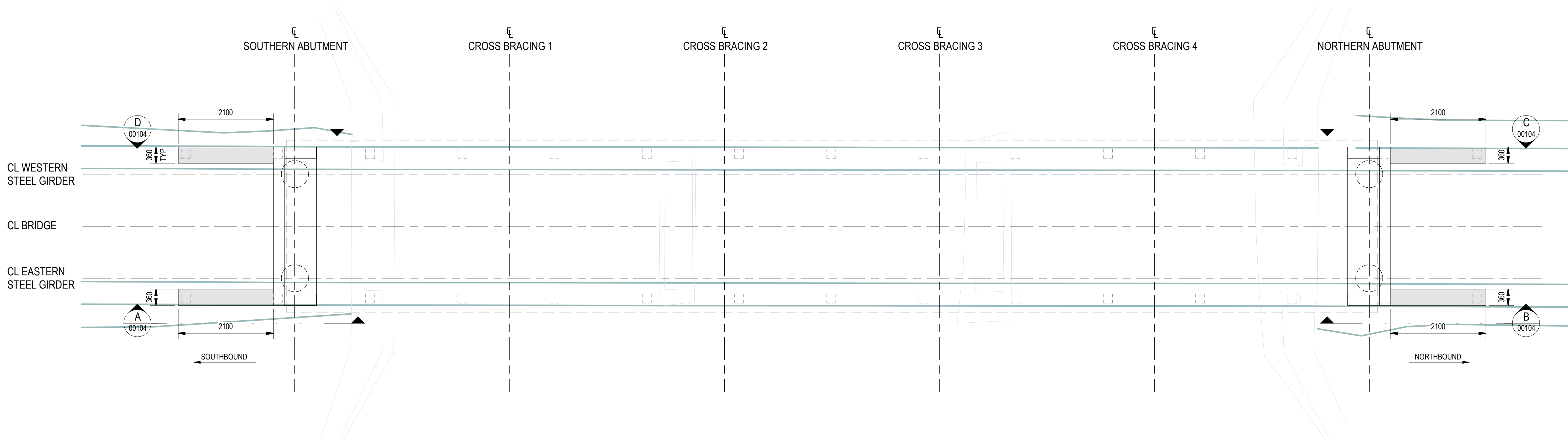
Drawing No.
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Size
A1

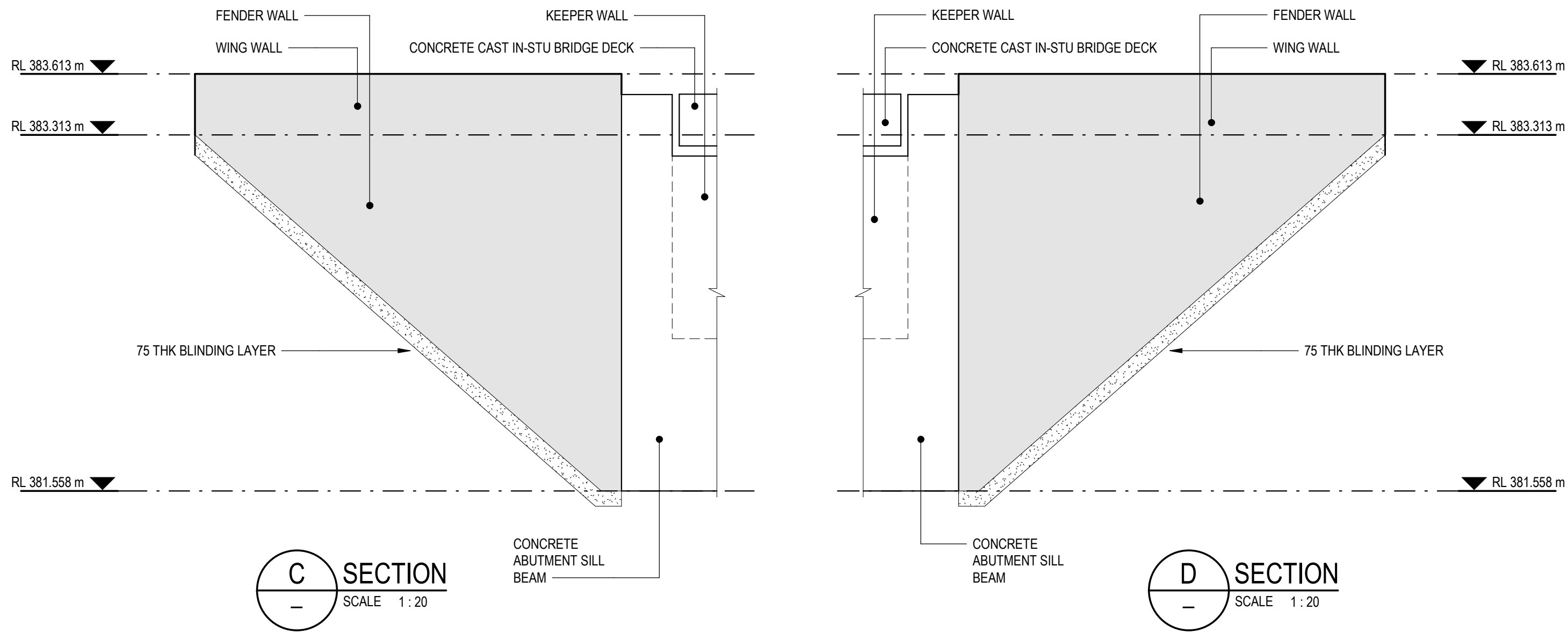
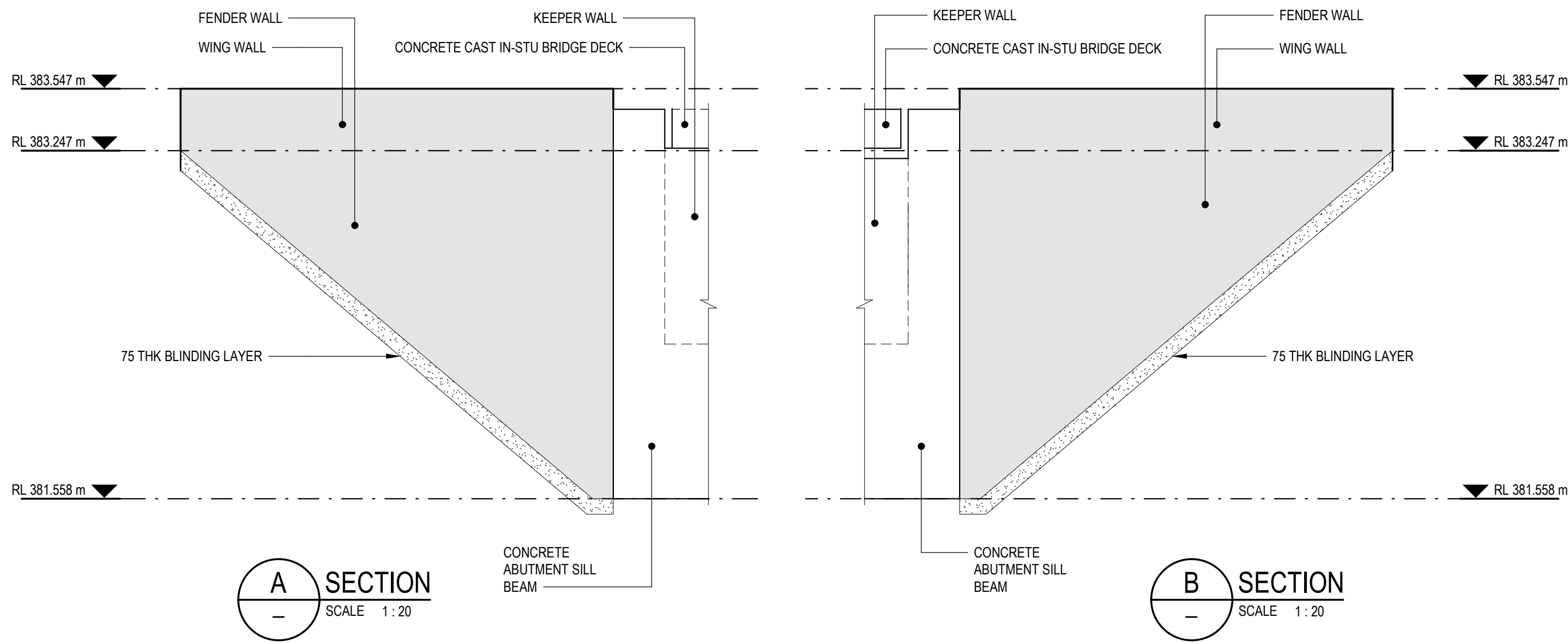
Rev
P01



- NOTES**
- FOR GENERAL NOTES REFER TO DRGS. 00010.
 - WEBFORGE HANDRAIL AT TOP WING WALLS NOT SHOWN. REFER TO DETAILS ON DRG. 00311.

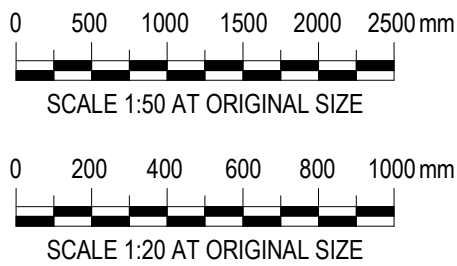


ANGAS CREEK WING WALLS LAYOUT PLAN
SCALE 1:50



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P01	90% DESIGN	AR	LR	07.06.24
Rev	Description	Checked	Approved	Date
Author	A. PERNA	Drafting Check	L. PARRY	
Designer	C. NGUYEN	Design Check	A. RITCHIE	



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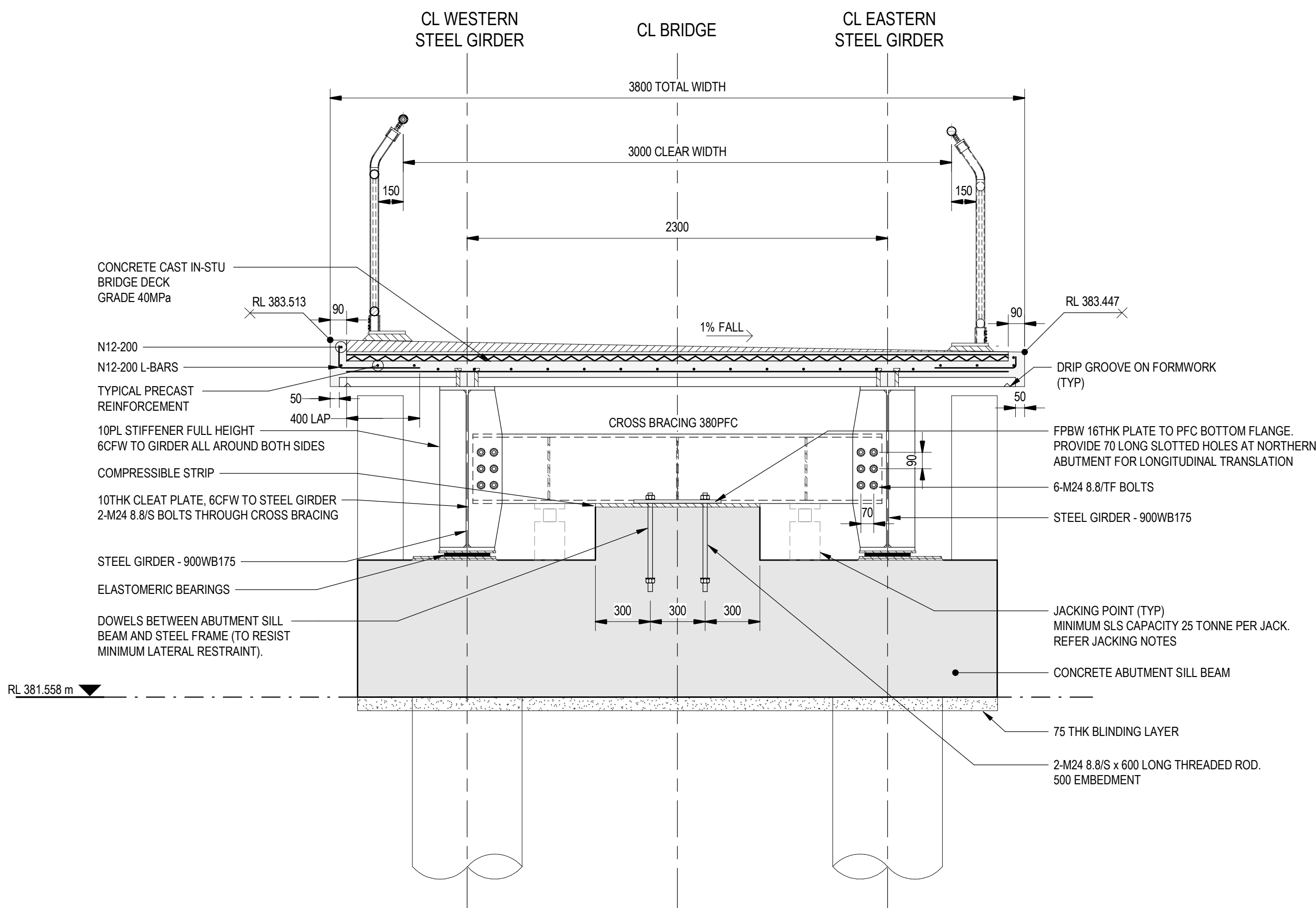
Project No.
12638597

Client **ADELAIDE HILLS COUNCIL**
Project **AMY GILLETT BIKEWAY EXTENSION**
Status **PRELIMINARY**

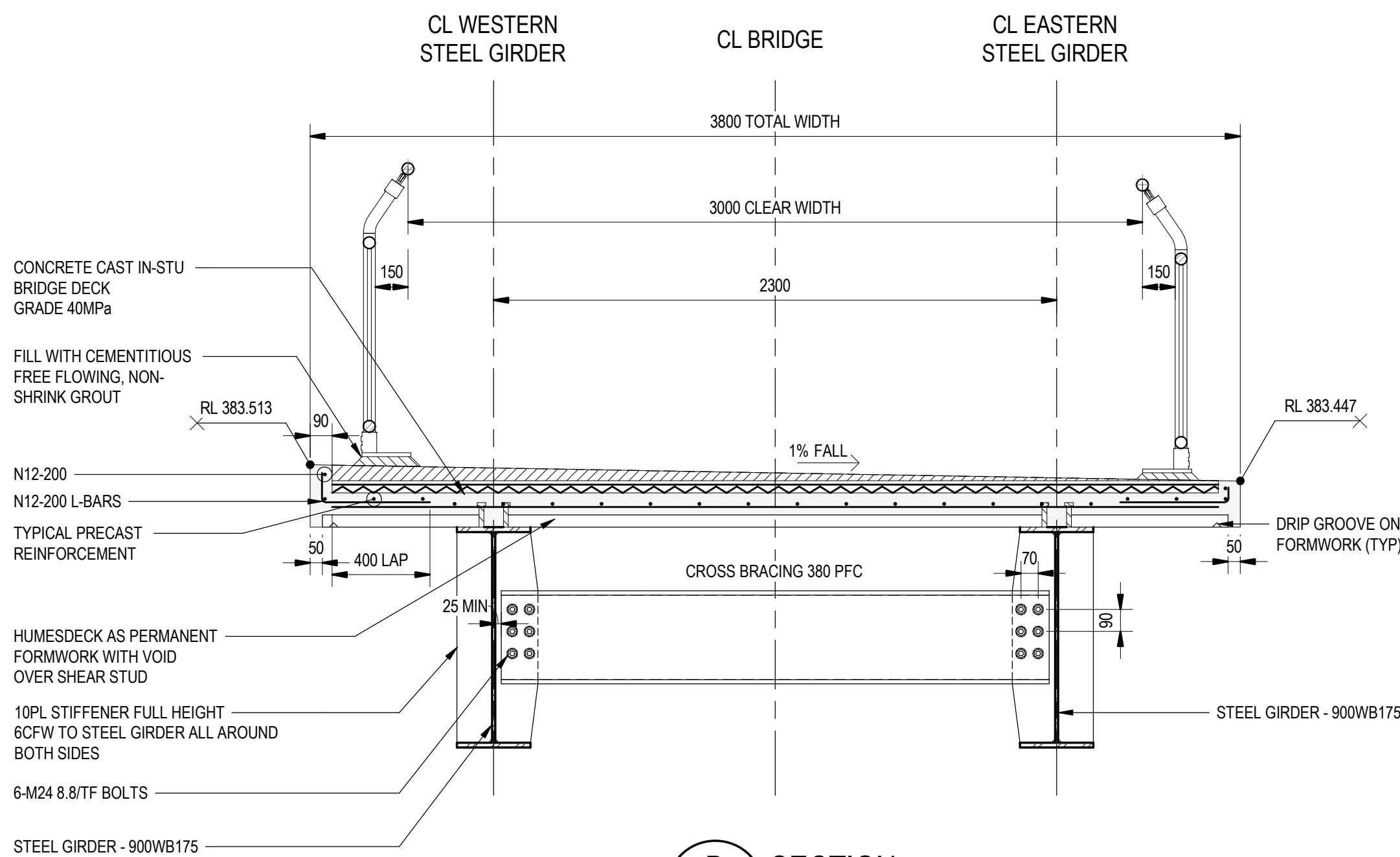
Drawing Title
**ANGAS CREEK BRIDGE WING WALLS
LAYOUT PLAN & SECTION**

Drawing No.
12638597-GHD-00-00-DRG-ST-00104

Size
A1
Rev
P01



A SECTION
00101 SCALE 1:20



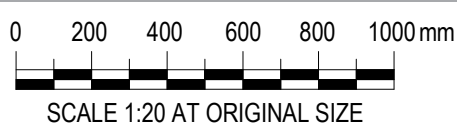
B SECTION
00101 SCALE 1:20

JACKING NOTES:

1. FOR GENERAL NOTES REFER TO DRGS. 00010.
2. MAXIMUM LIFT TO BE 2.5mm ABOVE BEARING.
3. JACK ALL BEARINGS AT ONE SUPPORT SIMULTANEOUSLY.
4. JACKING SHALL NOT BE PERFORMED DURING HIGH WINDS OR ADVERSE WEATHER.
5. NO LIVE LOAD ALLOWED ON BRIDGE DURING JACKING.
6. PROVIDE 50mm MIN. EDGE DISTANCE TO NEAREST CONCRETE EDGE FROM JACKING POINT.

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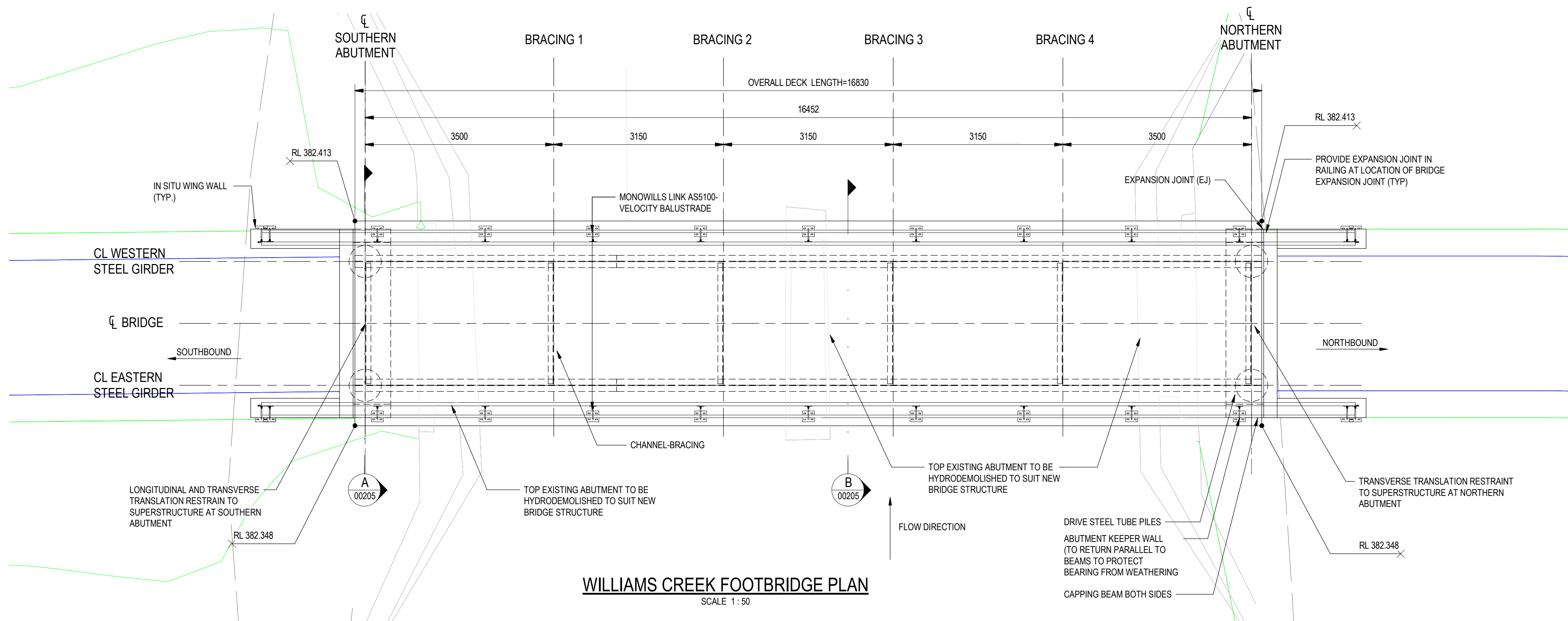
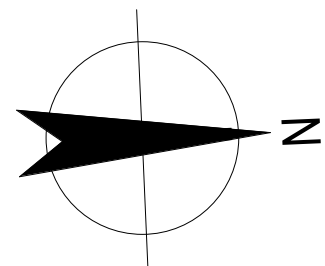
Client	ADELAIDE HILLS COUNCIL
Project	AMY GILLETT BIKEWAY EXTENSION
Status	PRELIMINARY

Drawing Title
ANGAS CREEK BRIDGE SECTIONS & DETAILS

Drawing No.
12638597-GHD-00-00-DRG-ST-00105

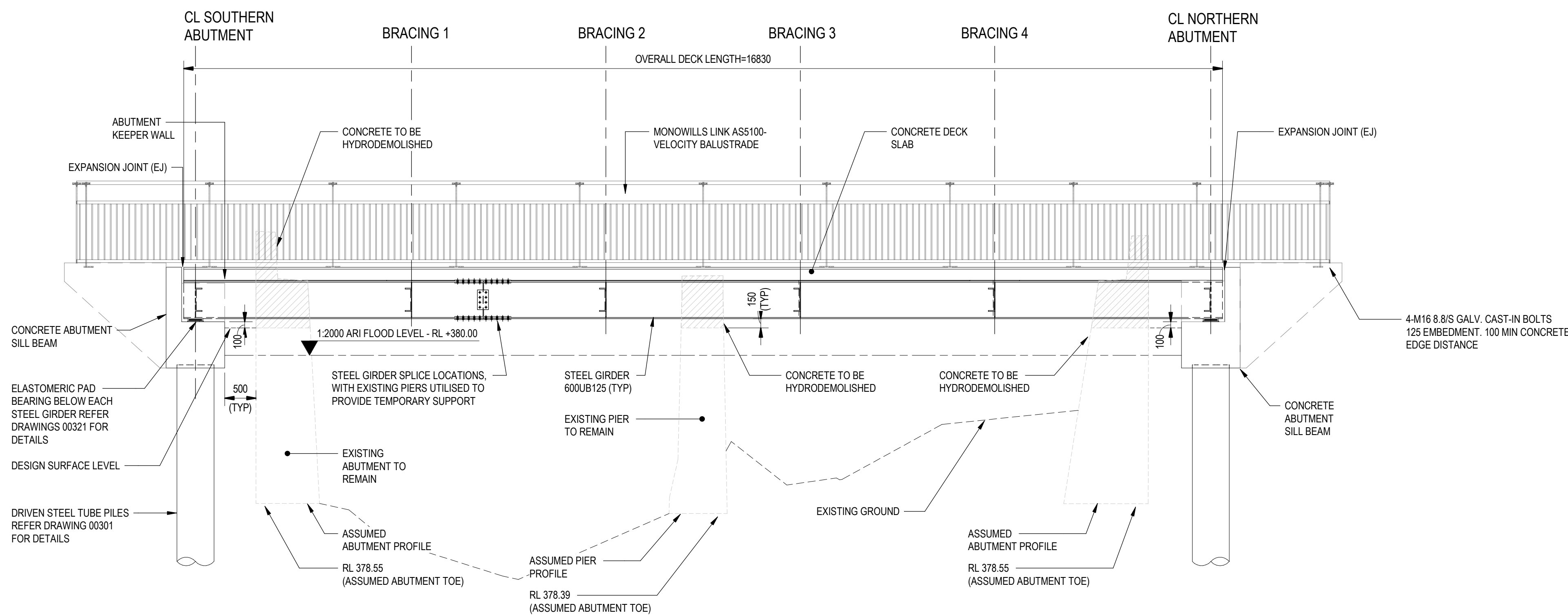
Size
A1

Rev
P01



WILLIAMS CREEK FOOTBRIDGE PLAN

SCALE 1:50



ELEVATION

SCALE 1:50

NOTES

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2. FOR GENERAL NOTES REFER TO DRGS 00010.

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Rev	Description	Checked	Approved	Date
Author	A. PERNA	Drafting Check	L. PARRY	
Designer	C. NGUYEN	Design Check	A. RITCHIE	

Plot Date: 7/06/2024 4:59:36 PM

File Name: Autodesk Docs://12638597-Amy Gillett Bridges/12638597-GHD-00-00-MDL-ST-00001-William Creek Footbridge.rvt



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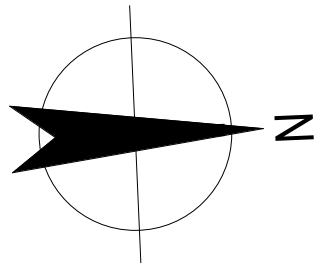
Client	ADELAIDE HILLS COUNCIL
Project	AMY GILLETT BIKEWAY EXTENSION
Status	PRELIMINARY

Drawing Title
WILLIAMS CREEK BRIDGE GENERAL ARRANGEMENT

Drawing No.
12638597-GHD-00-00-DRG-ST-00201

Size
A1

Rev
P01

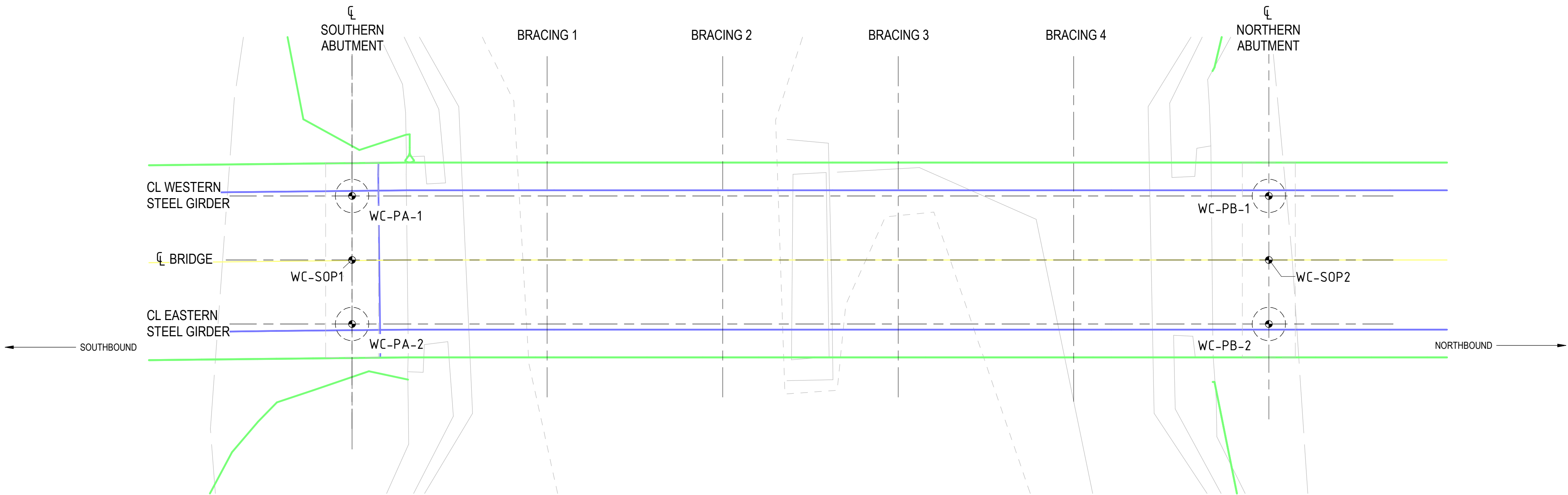


NOTES

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2. FOR GENERAL NOTES REFER TO DRGS 00010.

PILE NOTES:

1. ESTIMATED PILE TOE LEVEL CORRESPONDS TO THE PILE DESIGN GEOTECHNICAL STRENGTH ($R_d.g$) BEING EQUAL TO OR GREATER THAN THE DESIGN ACTION EFFECT (E_d) ie: $R_d.g \geq E_d$
2. THE PILE DESIGN GEOTECHNICAL STRENGTH ($R_d.g$) SHALL BE CALCULATED AS THE DESIGN ULTIMATE GEOTECHNICAL STRENGTH ($R_u.g$) MULTIPLIED BY THE GEOTECHNICAL STRENGTH FACTOR (ϕ_g) ACCORDING TO THE FOLLOWING EQUATION $R_d.g = \phi_g R_u.g$
3. THE GEOTECHNICAL STRENGTH REDUCTION FACTOR (ϕ) ADOPTED FOR DESIGN IS 0.7 IN ACCORDANCE WITH AS2159 FOR AXIAL COMPRESSION.
4. ONE PILE FROM EACH PILE GROUP HAS BEEN SELECTED (REFER SHEET No. 4111) FOR DYNAMIC LOAD TESTING WITH SIGNAL MATCHING ANALYSIS. THE TEST SHALL BE PERFORMED TO VERIFY THE DESIGN ULTIMATE GEOTECHNICAL STRENGTH ($R_d.g$) OF THE PILE.
5. THE GEOTECHNICAL DESIGN ASSUMES A GROUND WATER LEVEL OF 380.93m AUSTRALIAN HEIGHT DATUM. THE GROUNDWATER IS ASSUMED TO COINCIDE WITH THE MODELLED 0.05% AEP FLOOD LEVEL AT EACH BRIDGE. VARIATION IN GROUNDWATER LEVEL IS EXPECTED AND PERCHED WATER MAY BE PRESENT IN THE UPPER SOIL PROFILE. THE CONTRACTOR SHALL RECORD GROUNDWATER INFORMATION INCLUDING DEPTH AND ELEVATION AND PROVIDE THE RECORDED GROUNDWATER INFORMATION TO THE DESIGNER.



WILLIAMS CREEK FOOTBRIDGE PILE SETOUT PLAN

SCALE 1:50

BRIDGE PILE SCHEDULE - SOUTHERN...				
MARK	DIAMETER (mm)	EASTING (m)	NORTHING (m)	MINIMUM LENGTH OF PILE (mm)
WC-PA-1	600	313459.172	6143908.231	10000
WC-PA-2	600	313461.471	6143908.131	10000

BRIDGE SETOUT TABLE		
SOP MARK	EASTING (m)	NORTHING (m)
WC-SOP1	313460.322	6143908.181
WC-SOP2	313461.036	6143924.618

NOTE:
BRIDGE SET-OUT POINTS (SOP) PROVIDED AT INTERSECTION OF BRIDGE CENTRELINE AND CENTRELINE OF ABUTMENT

BRIDGE PILE SCHEDULE - NORTHERN...				
MARK	DIAMETER (mm)	EASTING (m)	NORTHING (m)	MINIMUM LENGTH OF PILE (mm)
WC-PB-1	600	313459.88	6143924.668	10000
WC-PB-2	600	313462.185	6143924.658	10000

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P01 90% FINAL DESIGN	AR	LR	07.06.24	
Rev Description	Checked	Approved	Date	
Author A. PERNA	Drafting Check	L. PARRY		
Designer C. NGUYEN	Design Check	A. RITCHIE		



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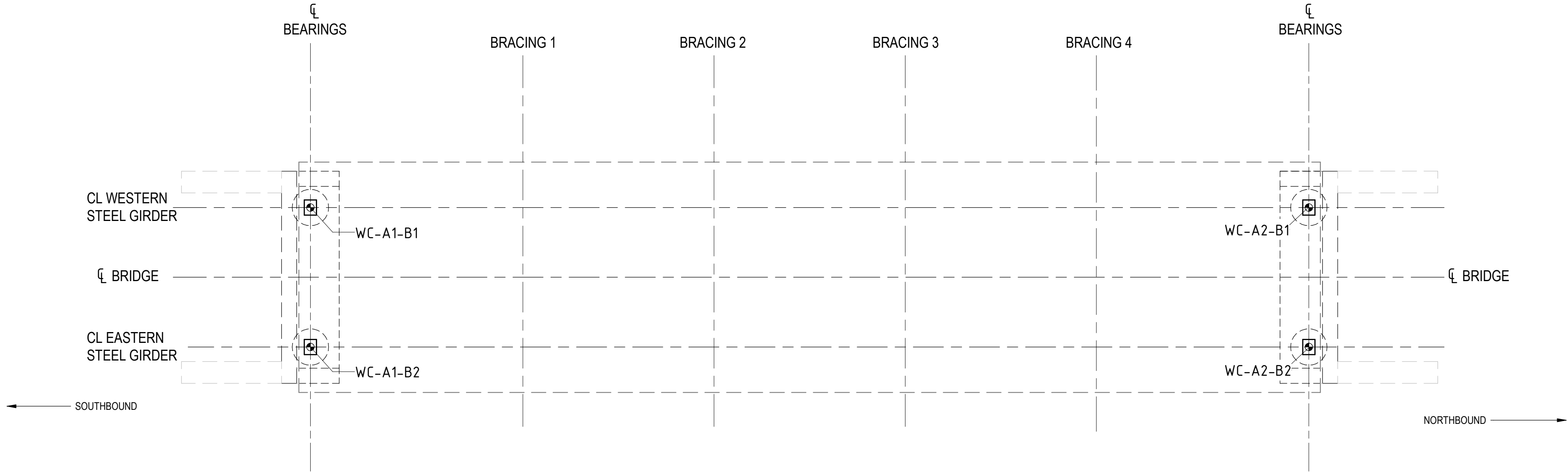
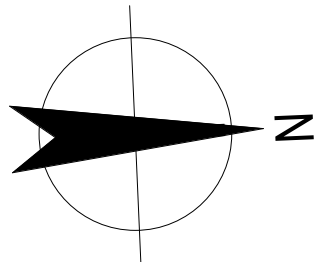
Client	ADELAIDE HILLS COUNCIL
Project	AMY GILLETT BIKEWAY EXTENSION
Status	PRELIMINARY

Drawing Title
WILLIAMS CREEK BRIDGE PILE SETOUT PLAN

Drawing No.
12638597-GHD-00-00-DRG-ST-00202

Size
A1

Rev
P01



WILLIAMS CREEK FOOTBRIDGE BEARING SETOUT PLAN
SCALE 1 : 50

BEARING SET-OUT TABLE		
SOP MARK	EASTING (m)	NORTHING (m)
WC-A1-B1	313459.181	6143908.422
WC-A1-B2	313461.479	6143908.322
WC-A2-B1	313459.886	6143924.668
WC-A2-B2	313462.185	6143924.568

NOTES

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5. STEEL ATTACHMENT PLATES AND KEEPER PLATES SHALL BE HOT DIP GALVANISED PRIOR TO ASSEMBLY.
6. TOLERANCE FOR MACHINING OF STEEL PLATES IS $\pm 0.5\text{mm}$.
7. MACHINE ALL STEEL PLATE FACES IN CONTACT WITH BEAMS.
8. ALL BEARINGS SHALL BE SET HORIZONTALLY IN ALL DIRECTIONS.
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10. THE SURFACE DIRECTLY BELOW THE GROUT PAD SHALL BE SCABBLED AND CLEANED TO REMOVE ALL LOOSE MATERIAL. THE AREA TO BE SCABBLED SHALL BE EQUAL TO THE BASE OF THE FINAL GROUT PAD LESS 20mm ON ALL SIDES.

NOT FOR CONSTRUCTION

P01	90% FINAL DESIGN	AR	LR	07.06.24
Rev	Description	Checked	Approved	Date
Author	A. PERNA	Drafting Check	L. PARRY	
Designer	C. NGUYEN	Design Check	A. RITCHIE	



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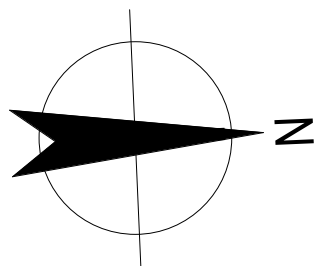
Client	ADELAIDE HILLS COUNCIL
Project	AMY GILLETT BIKEWAY EXTENSION
Status	PRELIMINARY

Drawing Title
WILLIAMS CREEK BRIDGE BEARING SETOUT PLAN

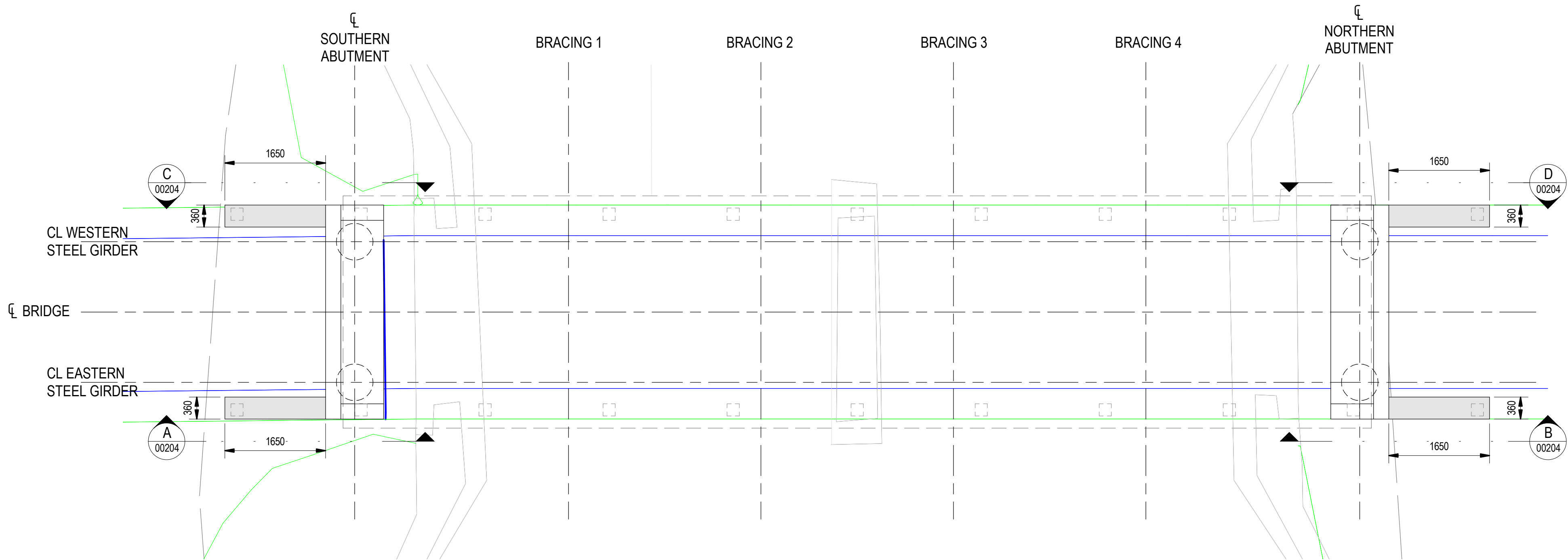
Drawing No.
12638597-GHD-00-00-DRG-ST-00203

Size
A1

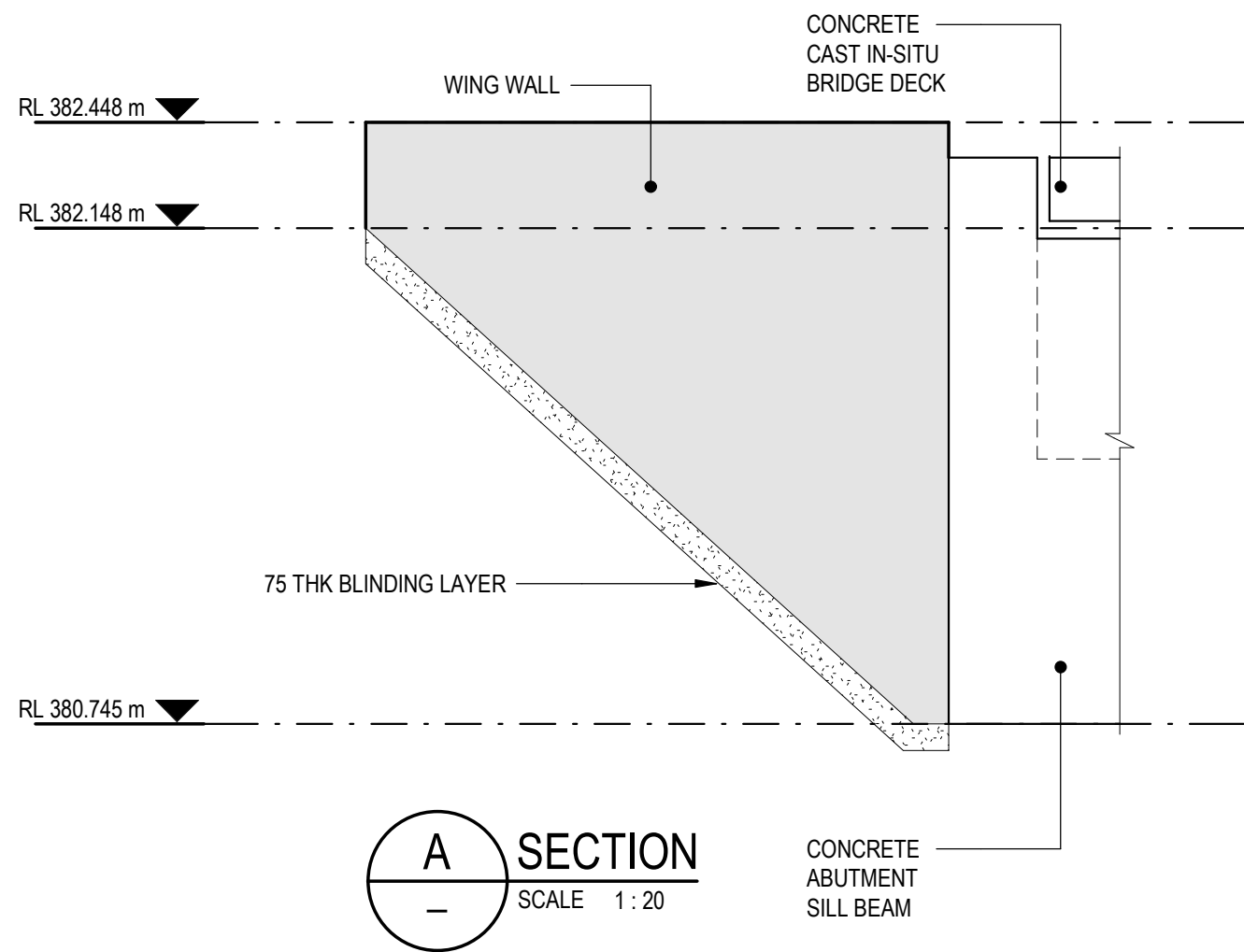
Rev
P01



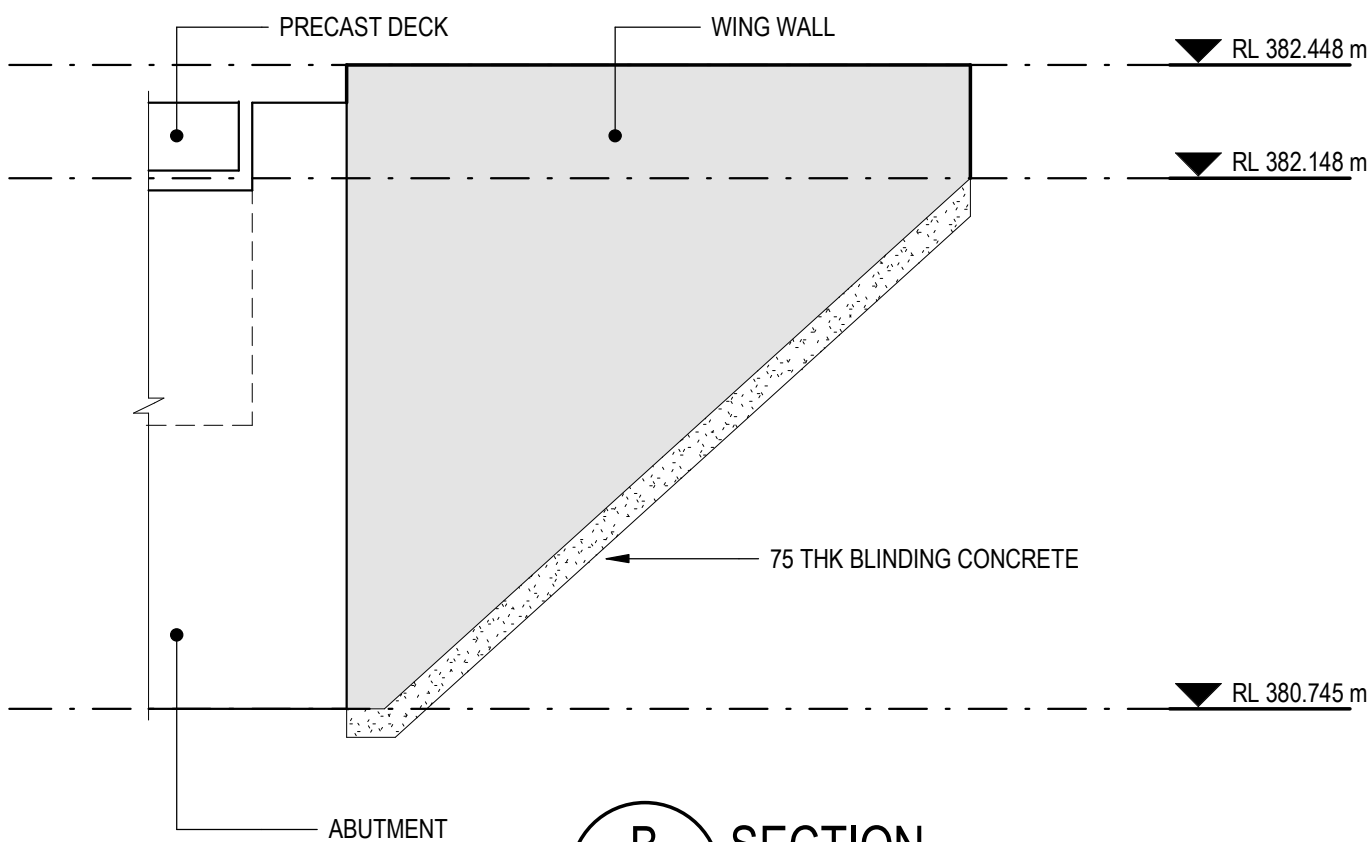
- NOTES**
- FOR GENERAL NOTES REFER TO SHEETS 00010 AND 00011.
 - WEBFORGE HANDRAIL AT TOP WING WALLS NOT SHOWN. REFER TO DETAILS ON SHEET 00311.



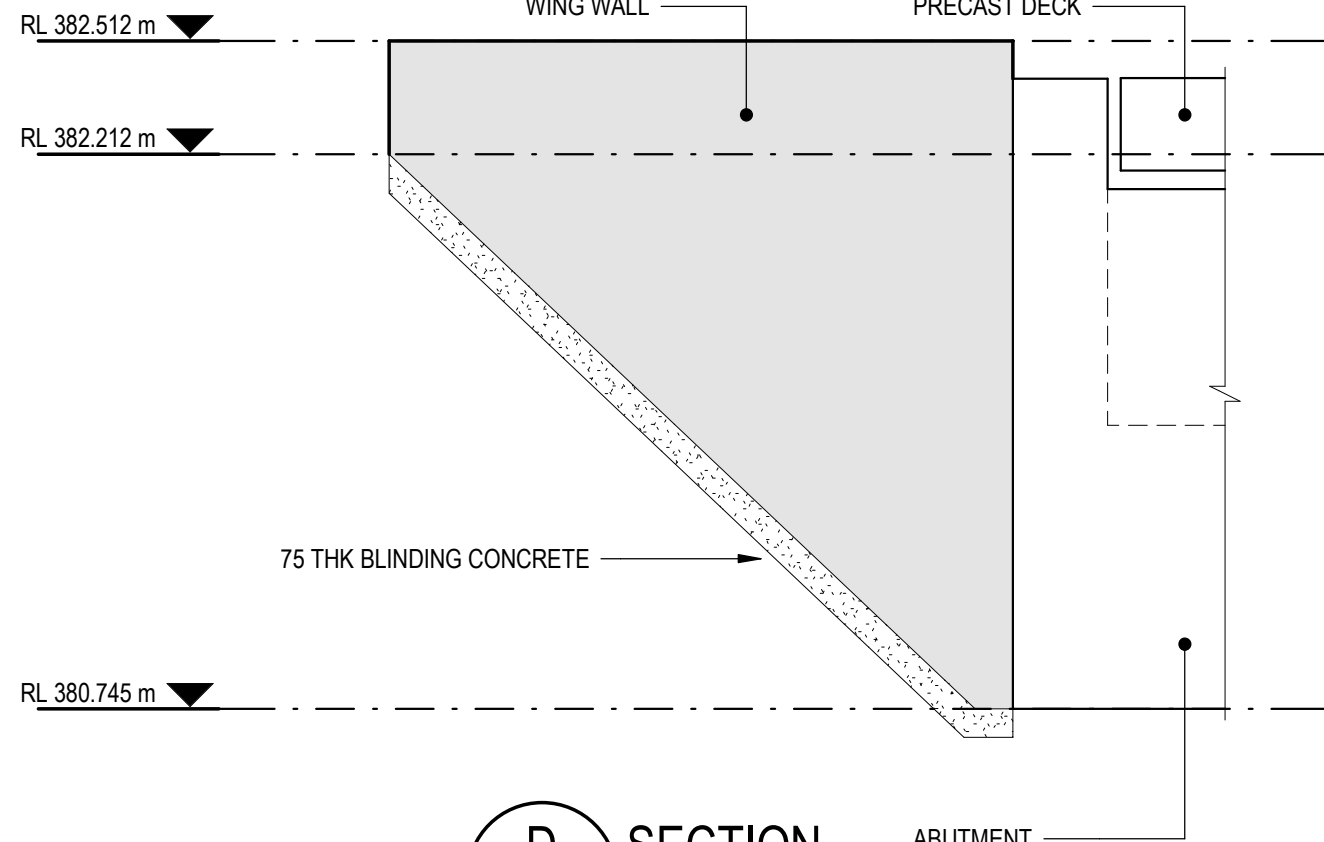
WILLIAMS CREEK WING WALLS LAYOUT PLAN
SCALE 1: 50



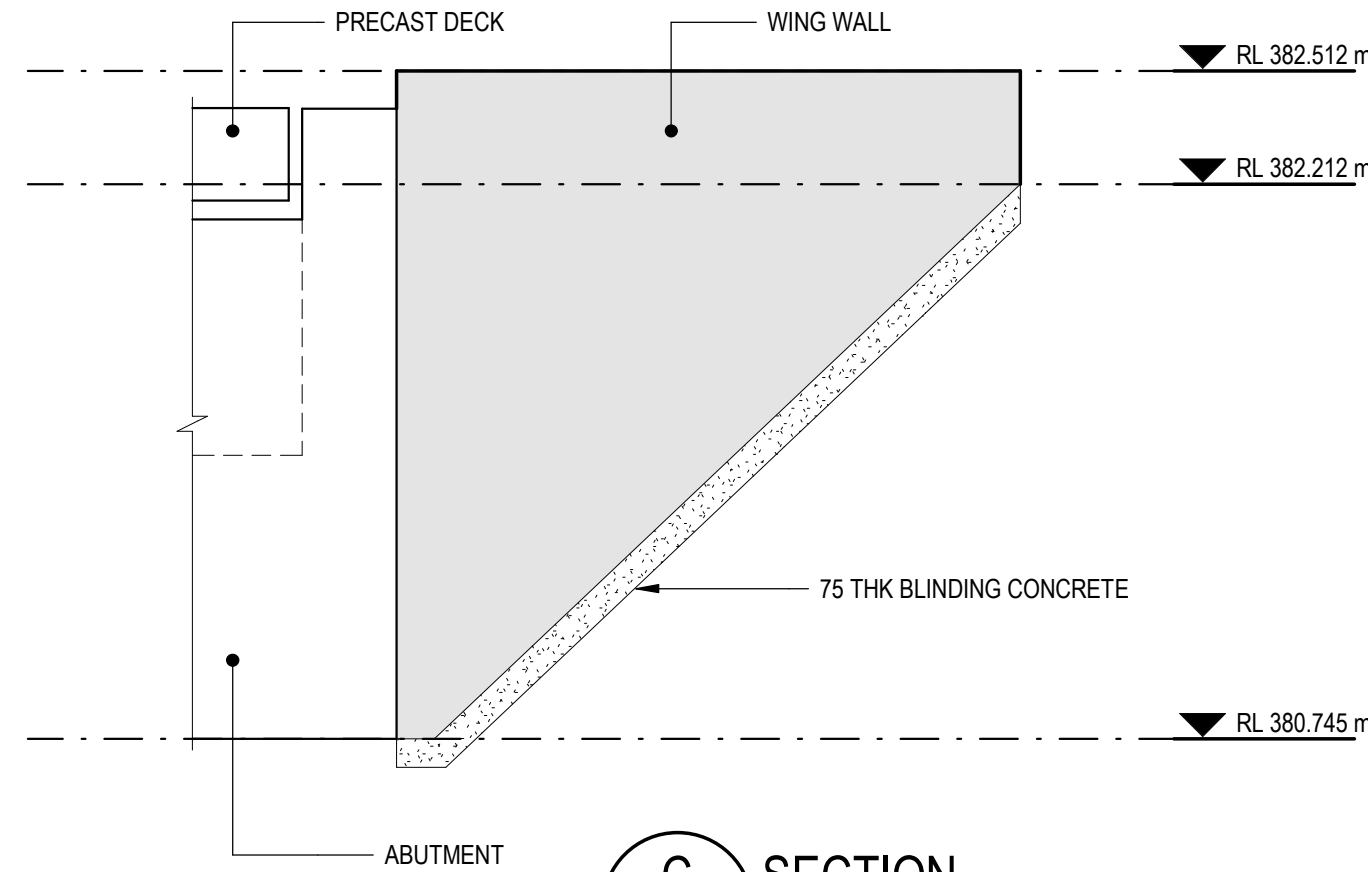
A SECTION
SCALE 1: 20



B SECTION
SCALE 1: 20



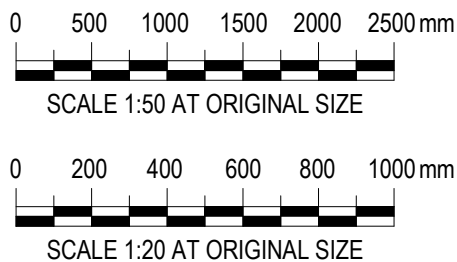
D SECTION
SCALE 1: 20



C SECTION
SCALE 1: 20

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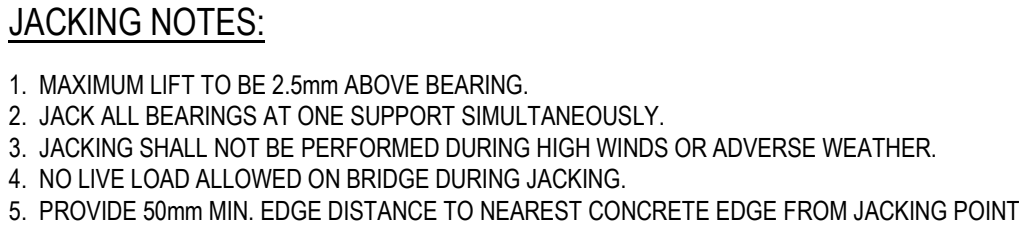
Project No.
12638597

Client **ADELAIDE HILLS COUNCIL**
Project **AMY GILLETT BIKEWAY EXTENSION**
Status **PRELIMINARY**

Drawing Title
WILLIAMS CREEK BRIDGE WING WALLS LAYOUT PLAN & SECTION

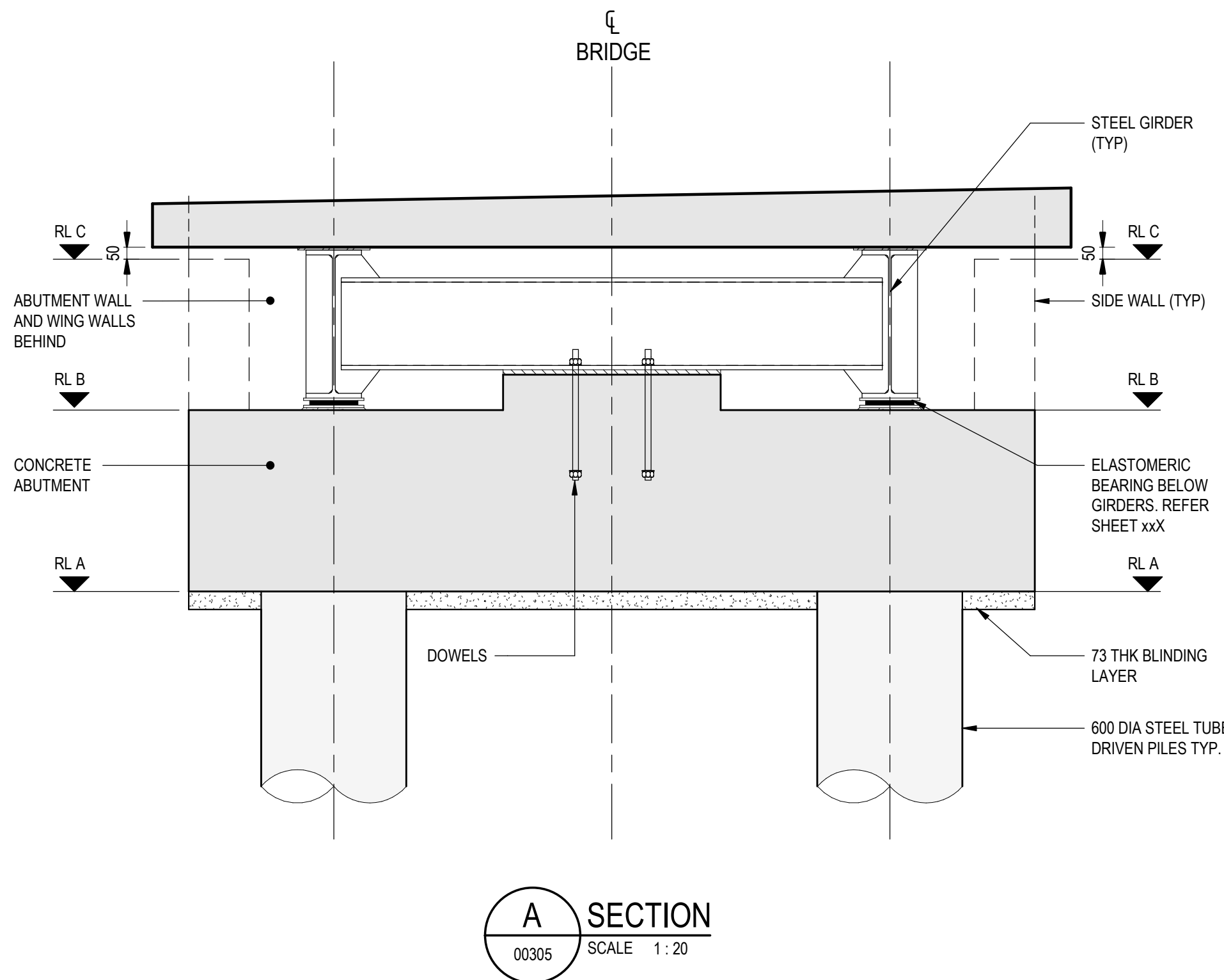
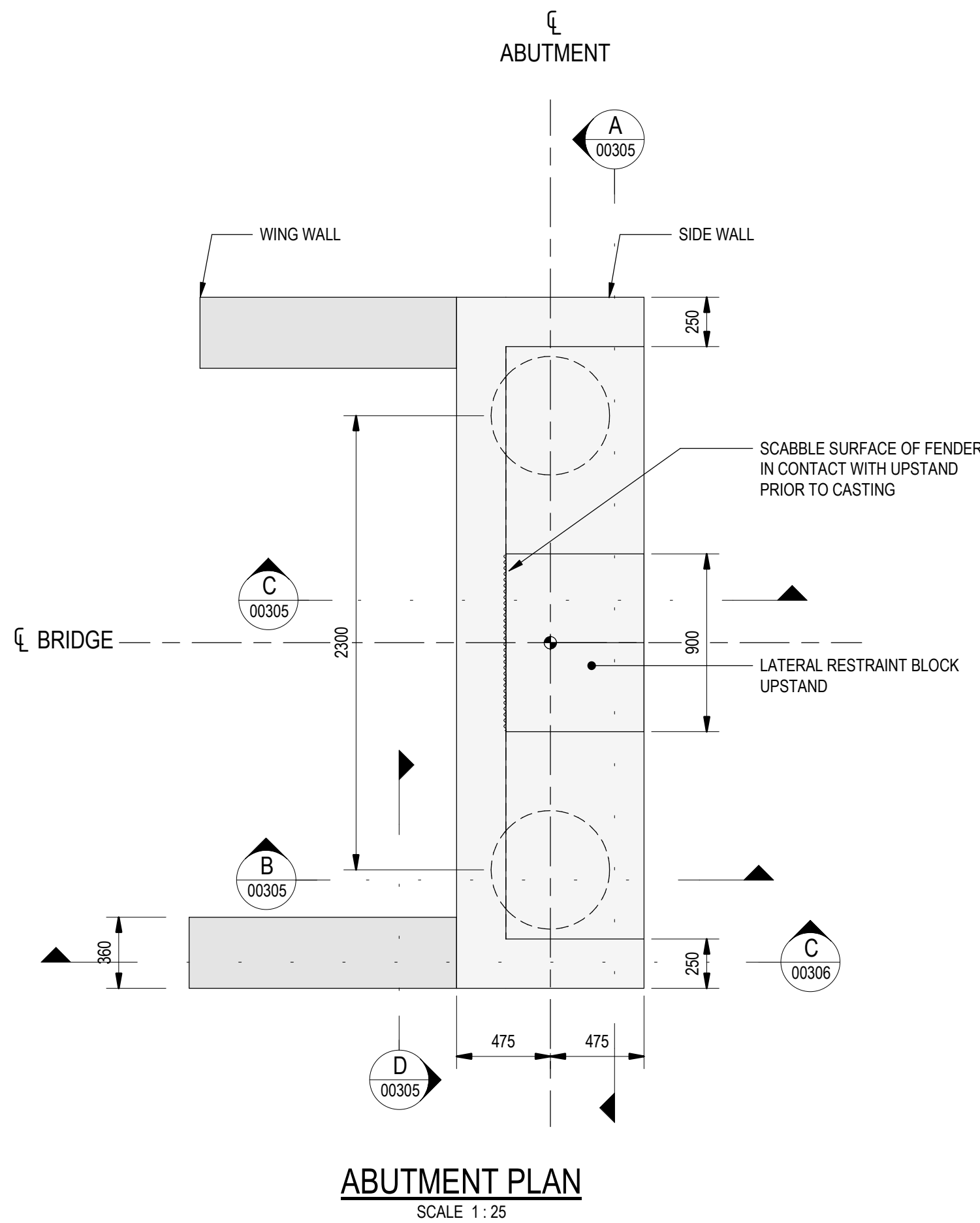
Drawing No.
12638597-GHD-00-00-DRG-ST-00204

Size
A1
Rev
P01

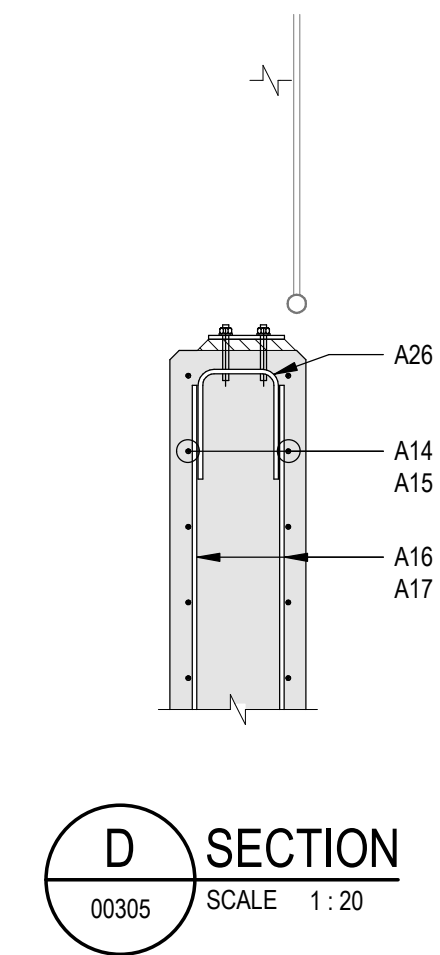
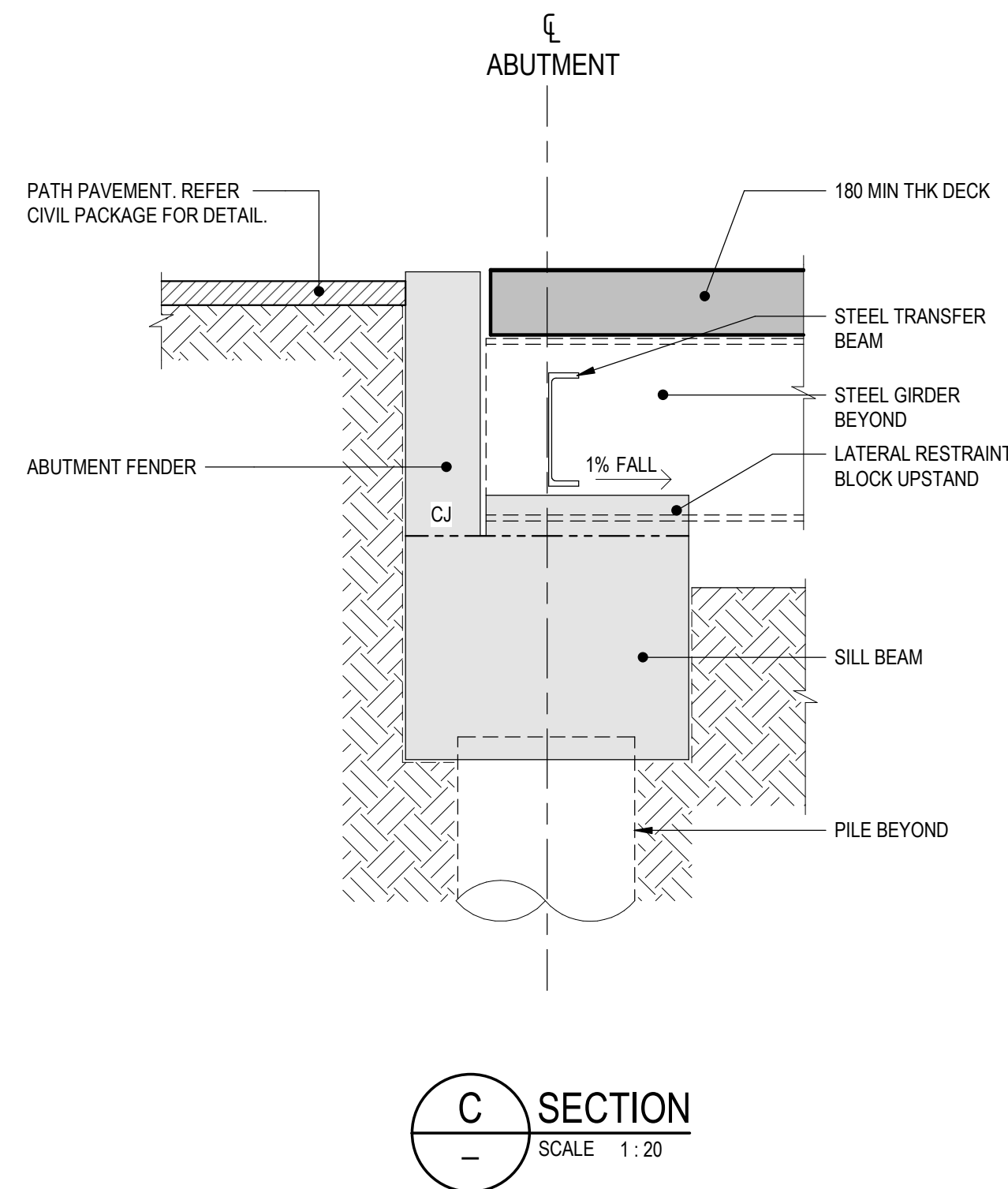
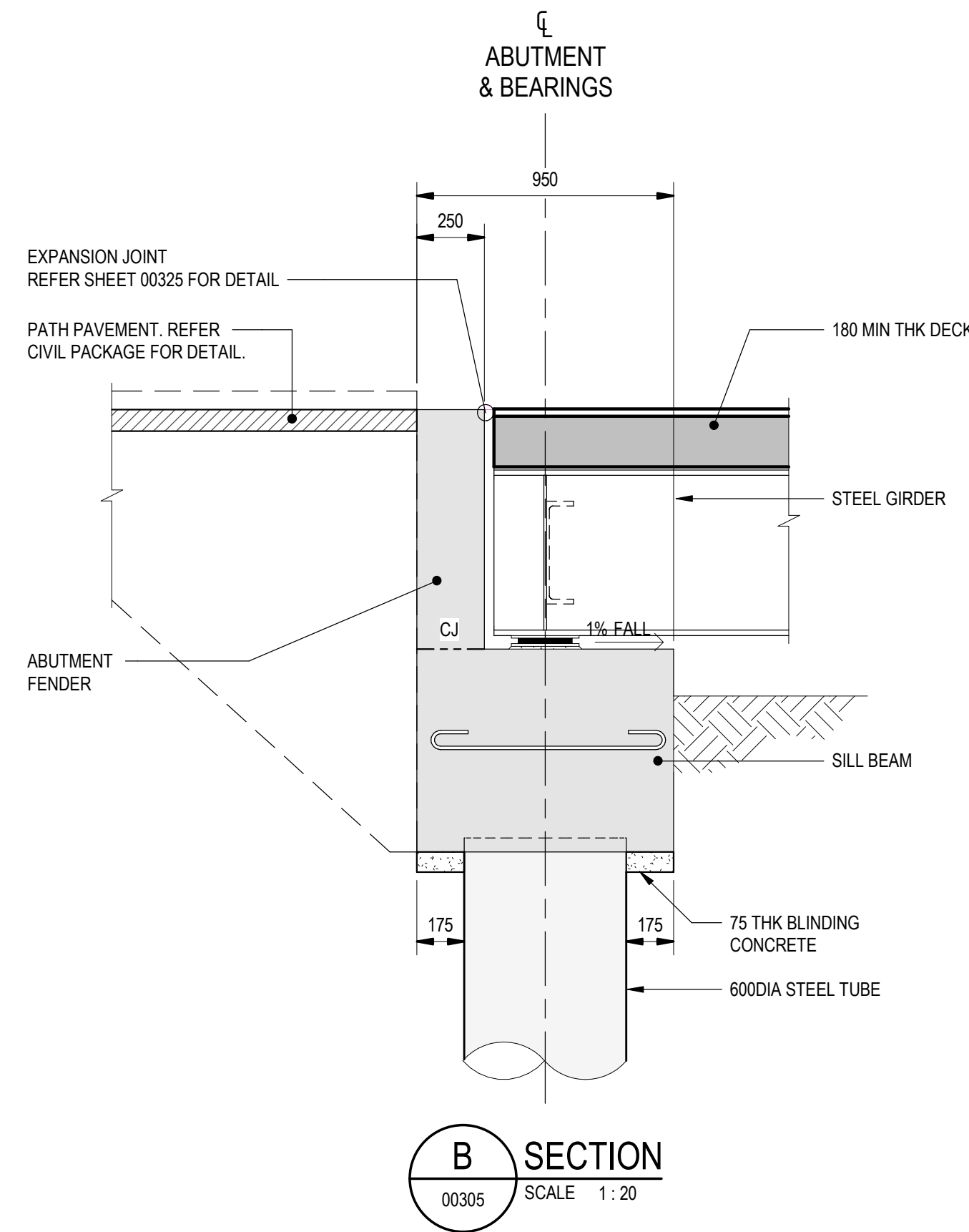


Technical drawing of a bridge deck cross-section. The drawing shows a rectangular deck with a central span of 2300 units. The total width is 3800 units, and the clear width is 3000 units. The deck is supported by two piers, labeled A and B. The deck is made of cast-in-place concrete, grade 40 MPa. The deck is supported by 600UB125 steel beams. The deck is filled with cementitious free flowing, non-shrink grout. The deck is supported by 10PL stiffeners full height 6CFW to girder all around both sides. The deck is supported by 380PFC steel beams. The deck is supported by 6-M24 8.8/TF bolts. The deck is supported by 25 MIN. The deck is supported by 70. The deck is supported by 50. The deck is supported by 1% FALL. The deck is supported by TOP OF DECK. The deck is supported by HUMESDECK AS PERMANENT FORMWORK WITH VOID OVER SHEAR STUD. The deck is supported by 600UB125. The deck is supported by 380PFC. The deck is supported by 6-M24 8.8/TF BOLTS. The deck is supported by 25 MIN. The deck is supported by 70. The deck is supported by 50. The deck is supported by 1% FALL. The deck is supported by TOP OF DECK.

		 SCALE 1:20 AT ORIGINAL SIZE		 Adelaide Hills COUNCIL		 Level 4, 211 Victoria Square Adelaide SA 5000 Australia GPO Box 2052 Adelaide SA 5001 T 61 8 8111 6600 F 61 8 8111 6699 E adlmail@ghd.com.au W www.ghd.com		 www.ghd.com		Client ADELAIDE HILLS COUNCIL		Drawing Title WILLIAMS CREEK BRIDGE SECTIONS & DETAILS		Size A1	
AR LR 07.06.24 Checked Approved Date						Project AMY GILLETT BIKEWAY EXTENSION				Status PRELIMINARY		Drawing No. 12638597-GHD-00-00-DRG-ST-00205		Rev P01	

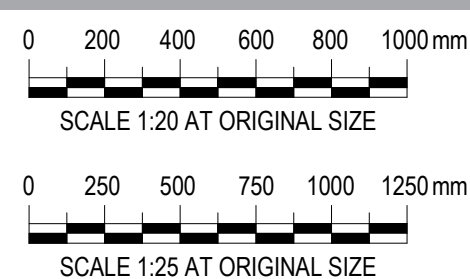


ABUTMENT LEVELS				
	ANGAS CREEK BRIDGE		WILLIAMS CREEK BRIDGE	
	ABUTMENT A	ABUTMENT B	ABUTMENT A	ABUTMENT B
RL 'A'	RL 380.745	RL 380.745	RL 381.558	RL 381.558
RL 'B'	RL 381.495	RL 381.495	RL 382.308	RL 382.308
RL 'C'	RL 382.118	RL 382.118	RL 383.208	RL 383.208



NOT FOR CONSTRUCTION

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Drafting Check				
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Project No.
12638597

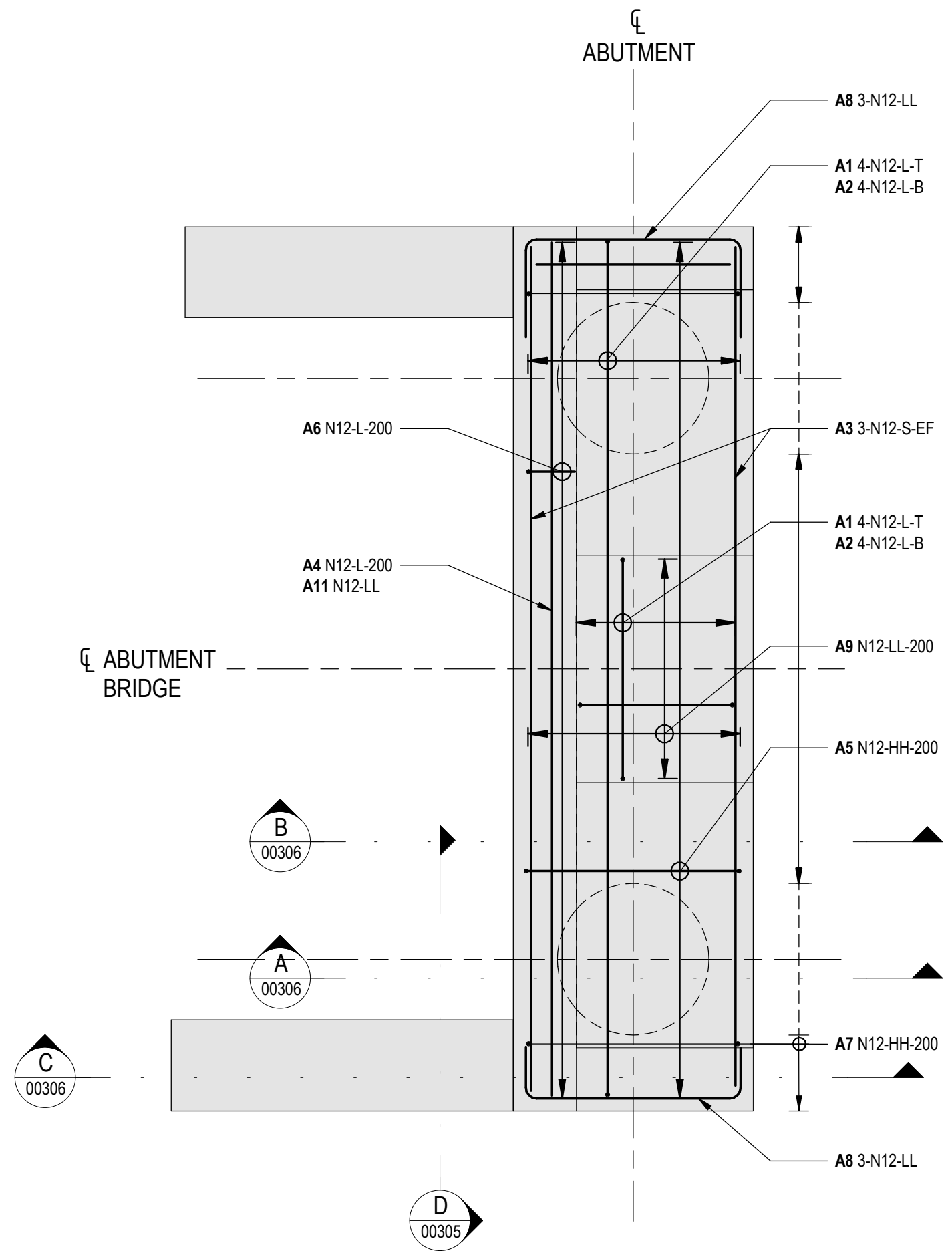
Client **ADELAIDE HILLS COUNCIL**
Project **AMY GILLETT BIKEWAY EXTENSION**
Status **PRELIMINARY**

Drawing Title
TYPICAL ABUTMENT CONCRETE DETAILS

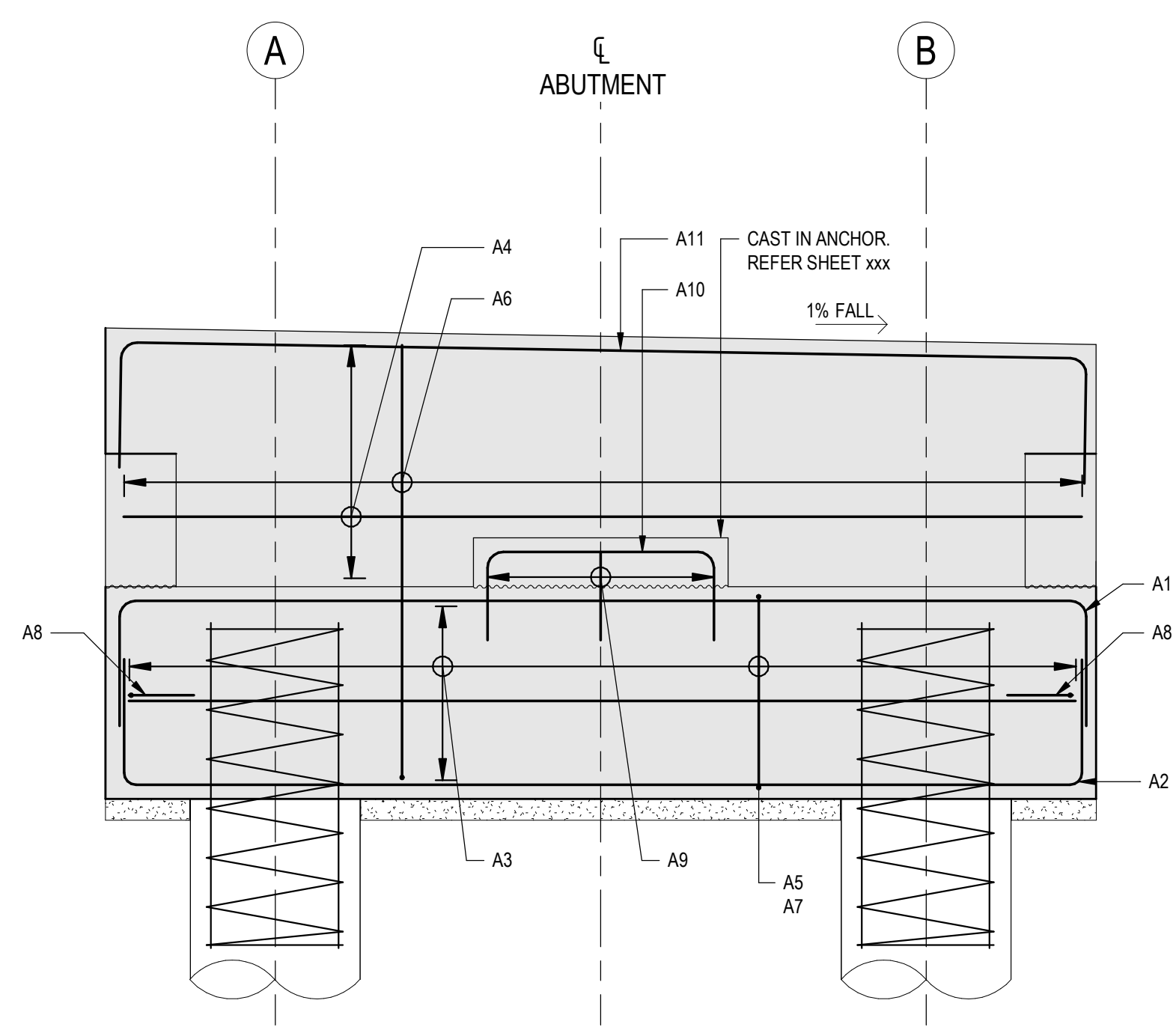
12638597-GHD-00-00-DRG-ST-00305

Size
A1

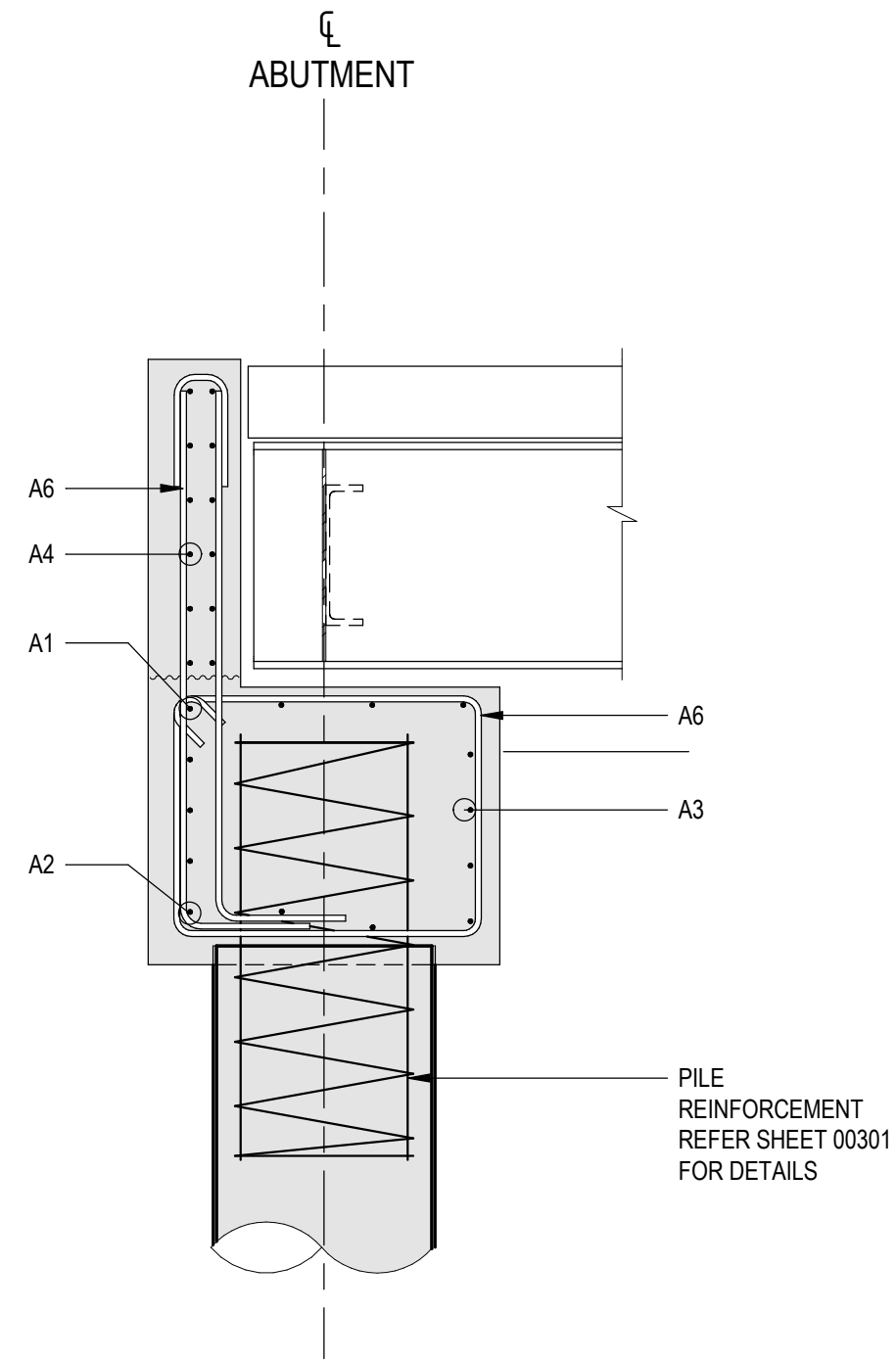
Rev
P01



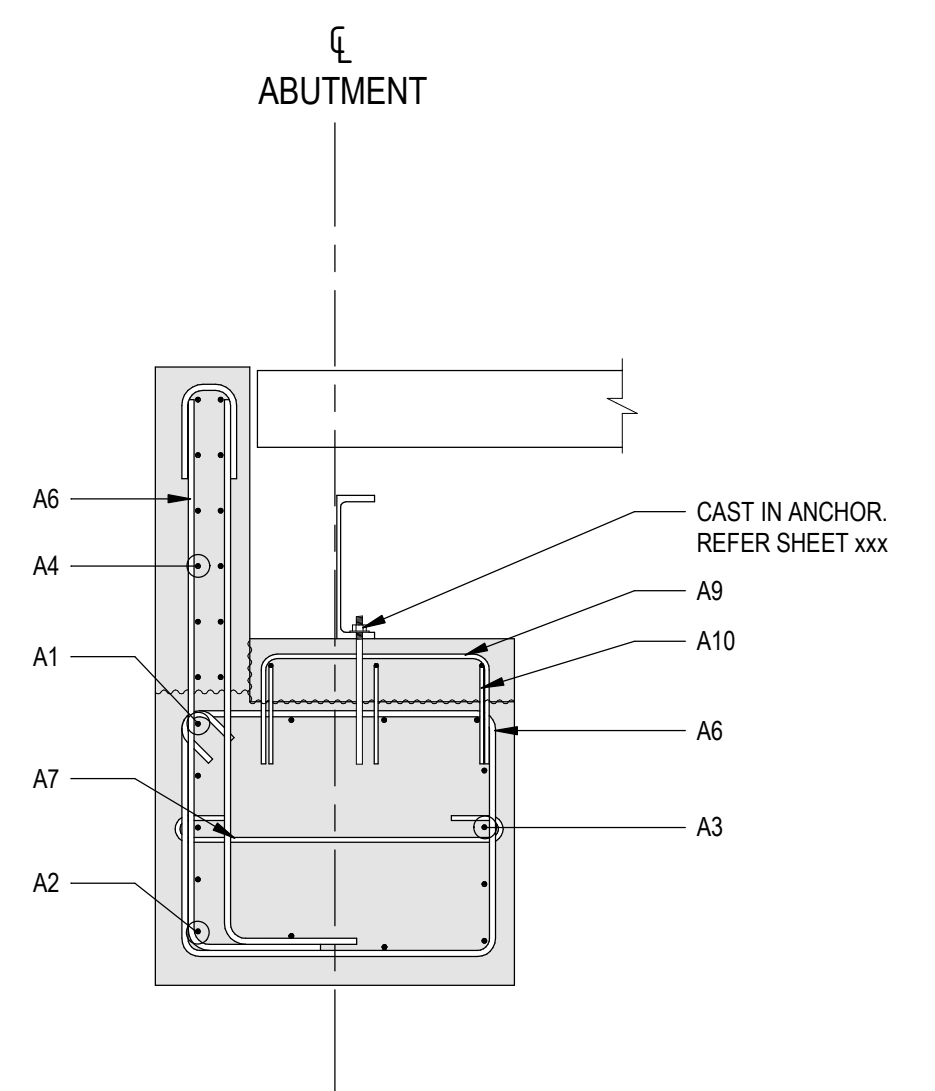
TYPICAL ABUTMENT REINFORCEMENT PLAN
SCALE 1:20



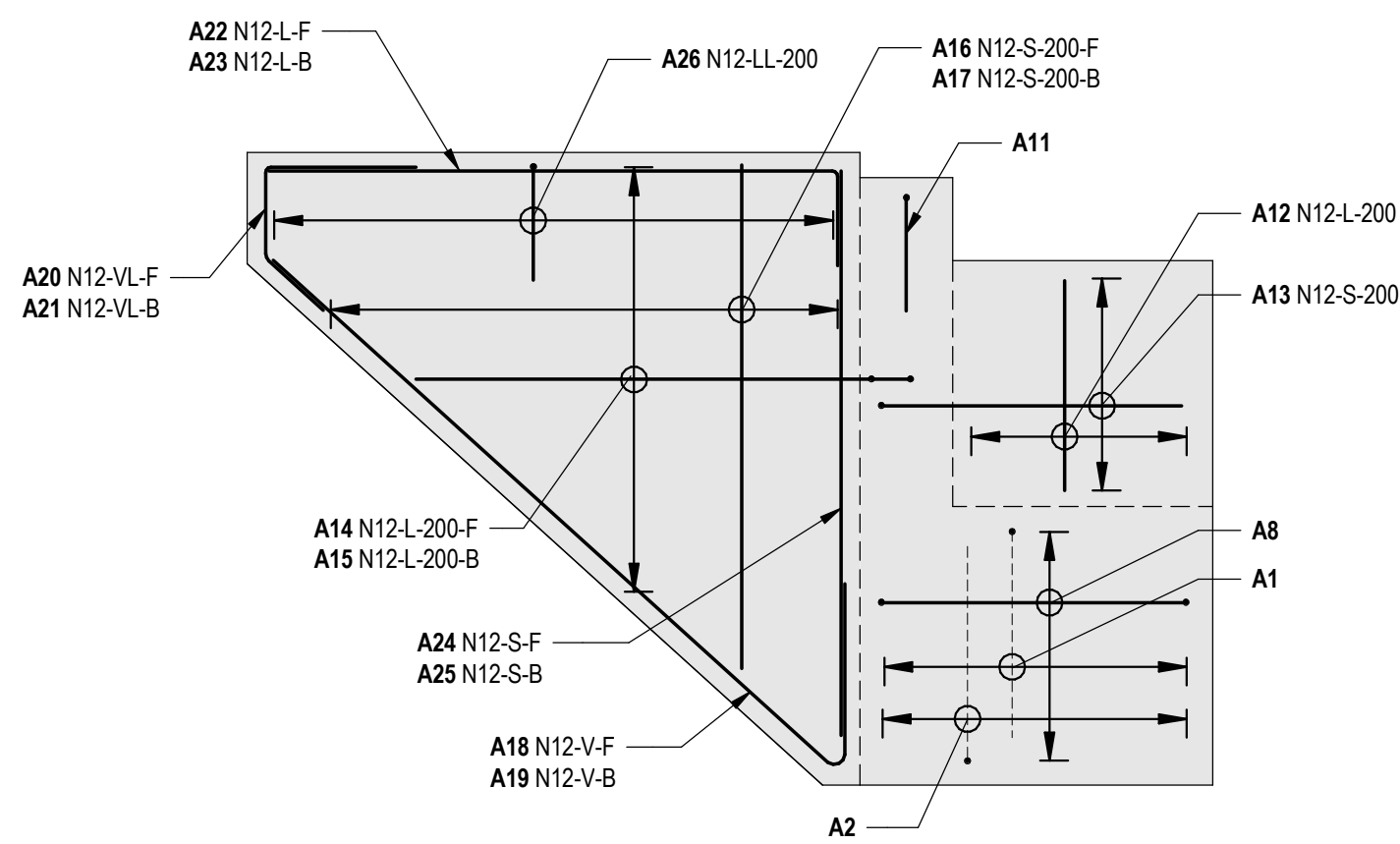
TYPICAL ABUTMENT ELEVATION
SCALE 1:20



A SECTION
00306 SCALE 1:20



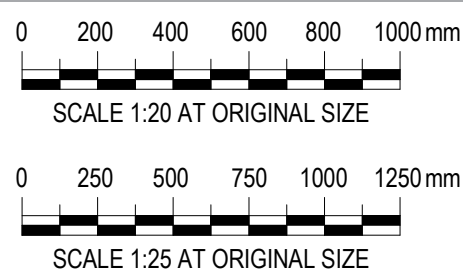
B SECTION
00306 SCALE 1:20



C SECTION
00305 SCALE 1:20

NOT FOR CONSTRUCTION

P01	90% FINAL DESIGN	AR	LR	07.06.24
Rev	Description	Checked	Approved	Date
Author	A. PERNA	Drafting Check	L. PARRY	
Designer	C. NGUYEN	Design Check	A. RITCHIE	



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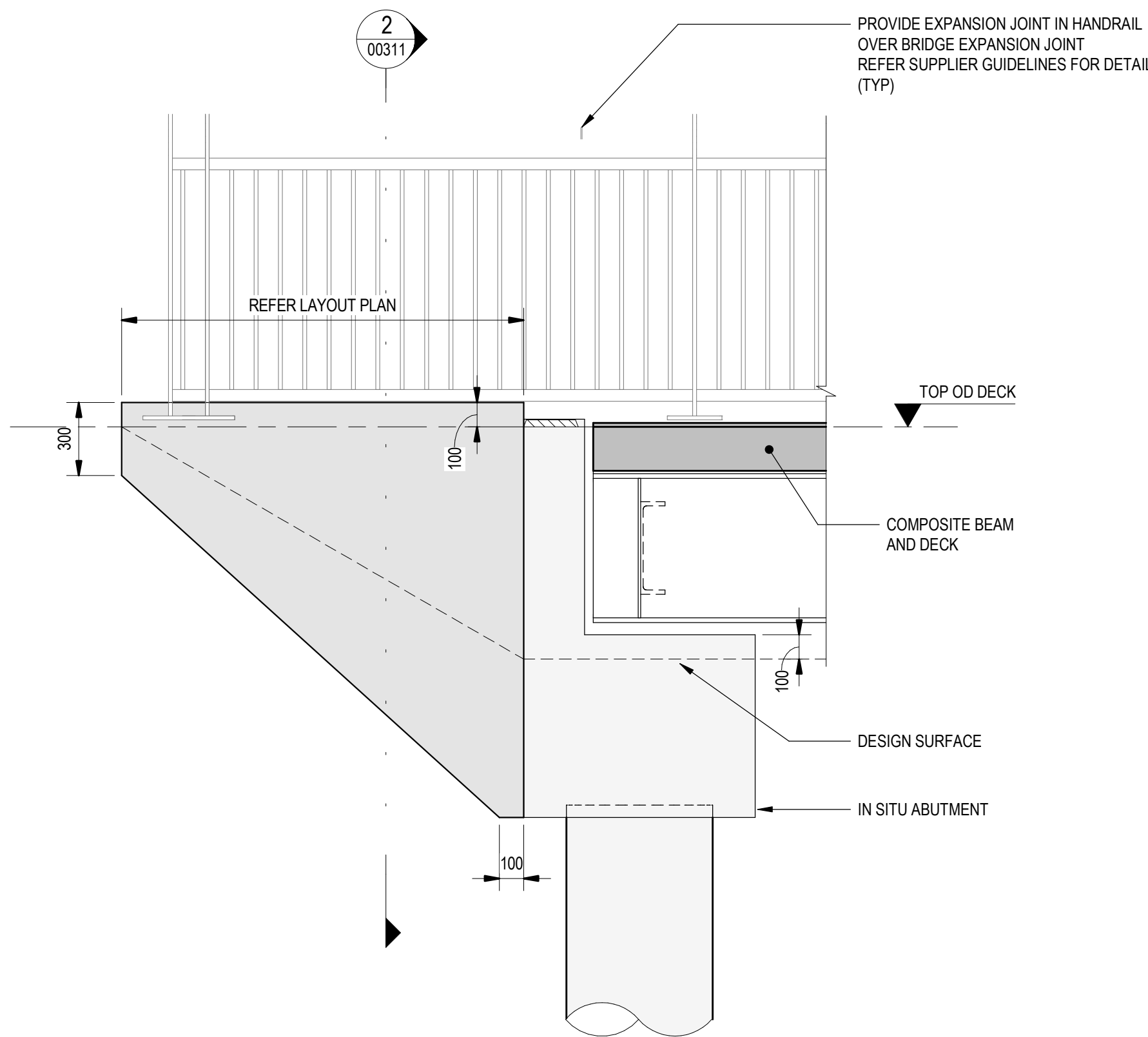
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Project	AMY GILLET BIKEWAY EXTENSION
Status	PRELIMINARY

Drawing Title
TYPICAL ABUTMENT REINFORCEMENT DETAILS

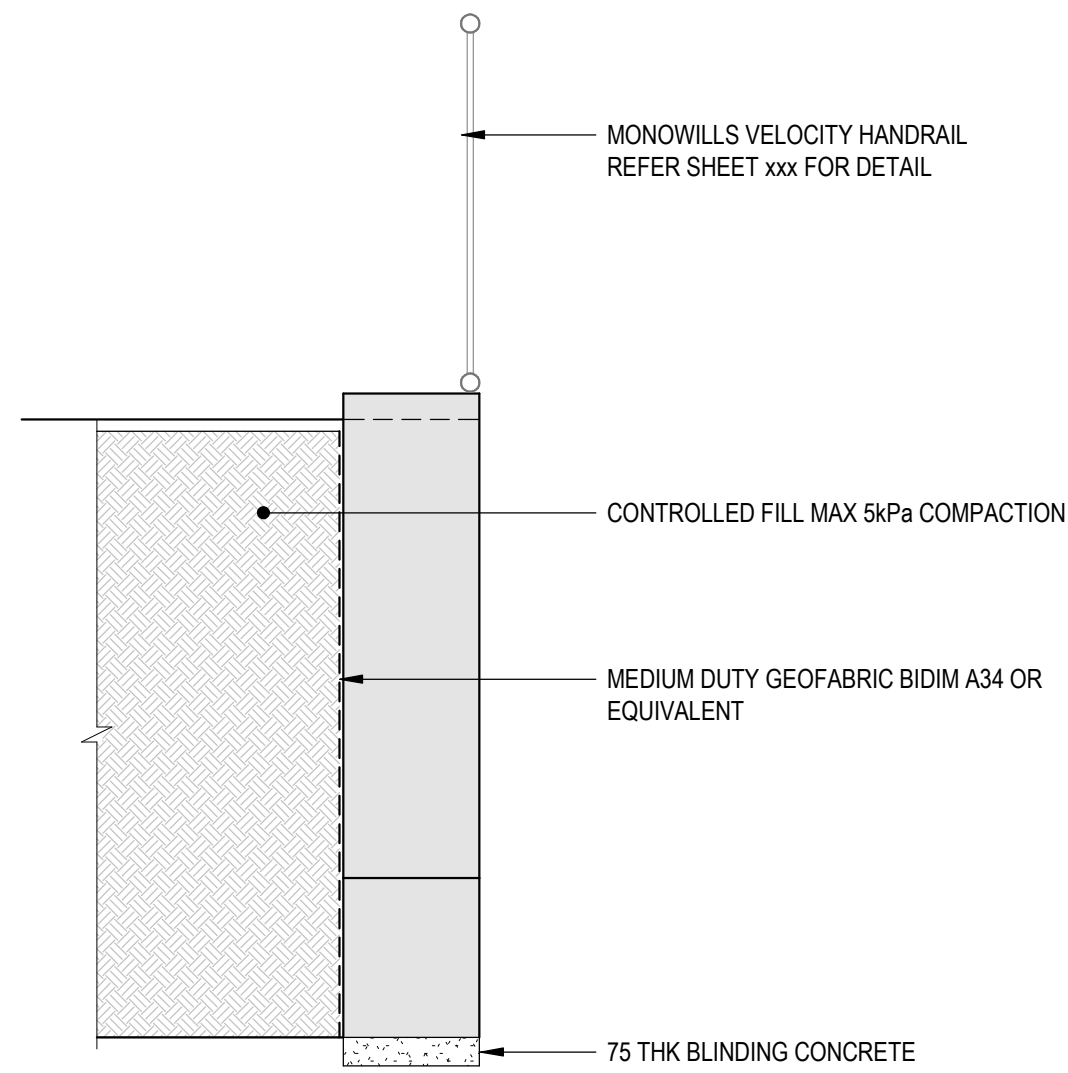
Drawing No.
12638597-GHD-00-00-DRG-ST-00306

Size
A1

Rev
P01



TYPICAL WING WALL ARRANGEMENT - ANGAS CREEK
SCALE 1 : 20



2 SECTION
SCALE 1 : 20

NOT FOR CONSTRUCTION

Rev	Description	Checked	Approved	Date
P01	90% FINAL DESIGN	AR	LR	07.06.24
Author	A. PERNA	Drafting Check	L. PARRY	
Designer	C. NGUYEN	Design Check	A. RITCHIE	



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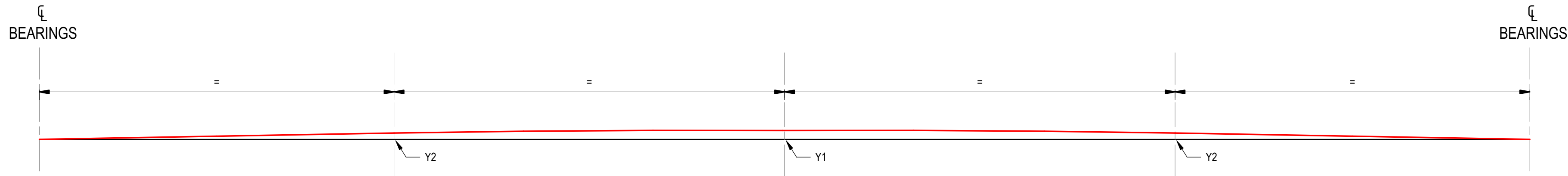
Client	ADELAIDE HILLS COUNCIL
Project	AMY GILLETT BIKEWAY EXTENSION
Status	PRELIMINARY

Drawing Title
TYPICAL WING WALLS DETAILS

12638597-GHD-00-00-DRG-ST-00311

Size
A1

Rev
P01



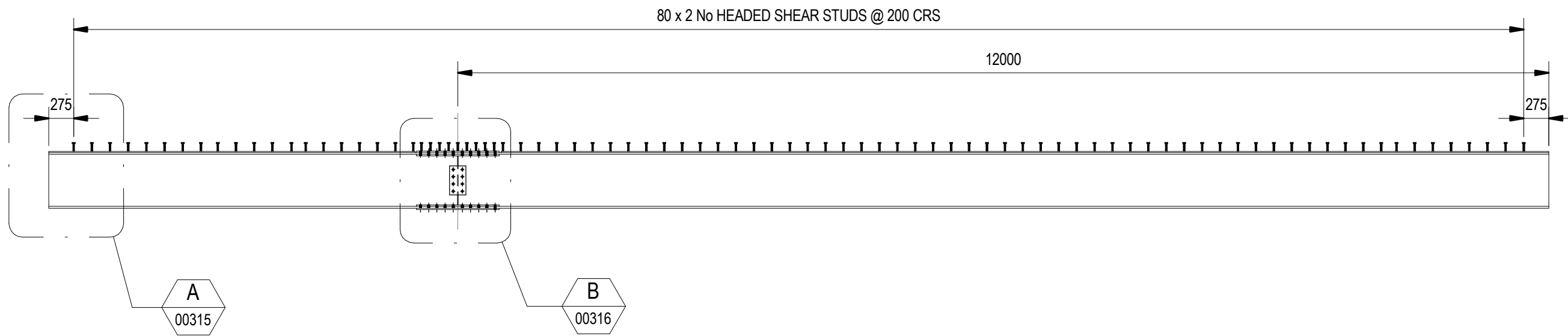
PRECAMBER DETAIL

SCALE 1:50

PRECAMBER (mm)		
BRIDGE	Y1	Y2
ANGAS CREEK BRIDGE	135	97
WILLIAMS CREEK BRIDGE	110	79

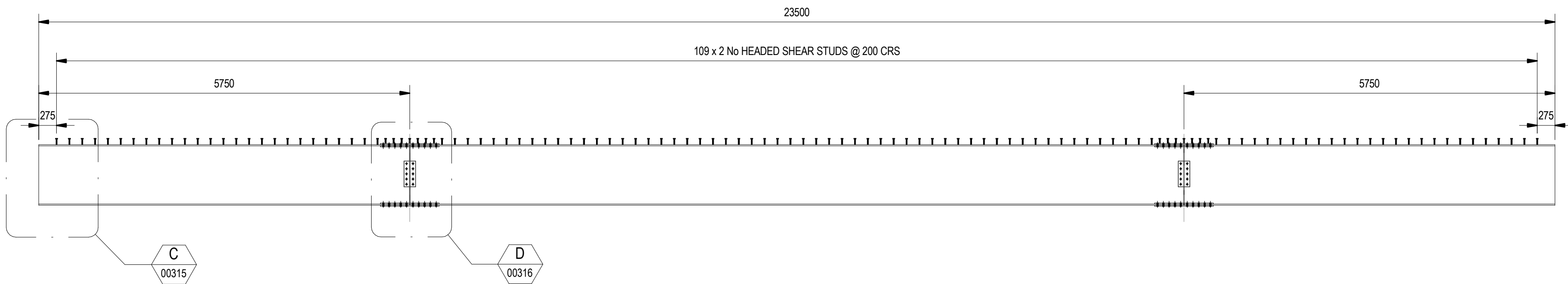
PRECAMBER AND STEEL WORK SETTING NOTE:

1. PRECAMBER DATA PREDICTS THE DEFLECTION OF THE STEEL GIRDER DUE TO THE SELF-WEIGHT OF THE STEEL WORK, PRECAST, CAST INSITU DECK CONCRETE, BARRIERS AND LONG TERM CREEP AND SHRINKAGE.



WILLIAM CREEK FOOTBRIDGE BEAM ELEVATION

SCALE 1:50

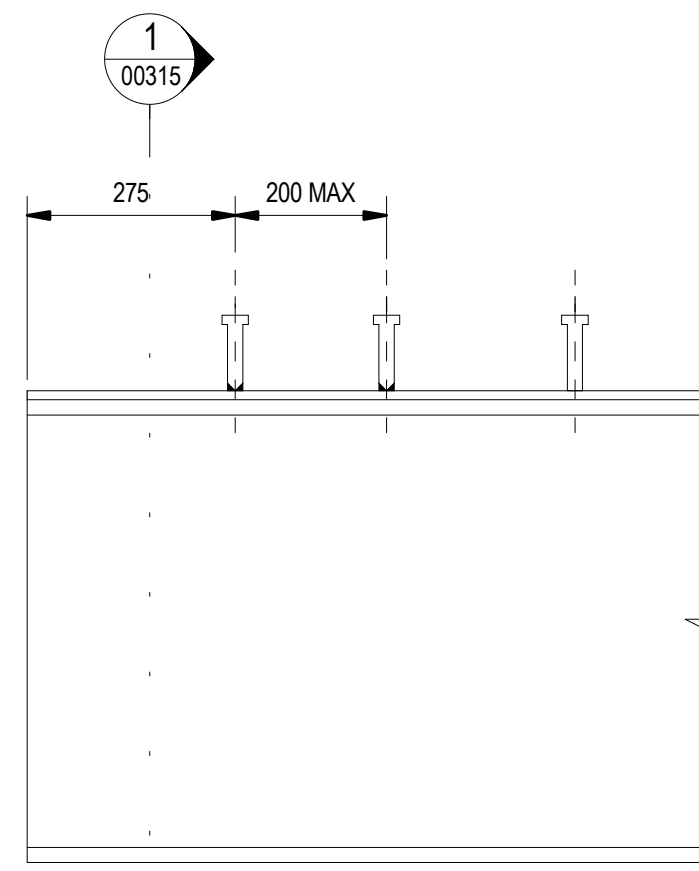


ANGAS CREEK FOOTBRIDGE BEAM ELEVATION

SCALE 1:50

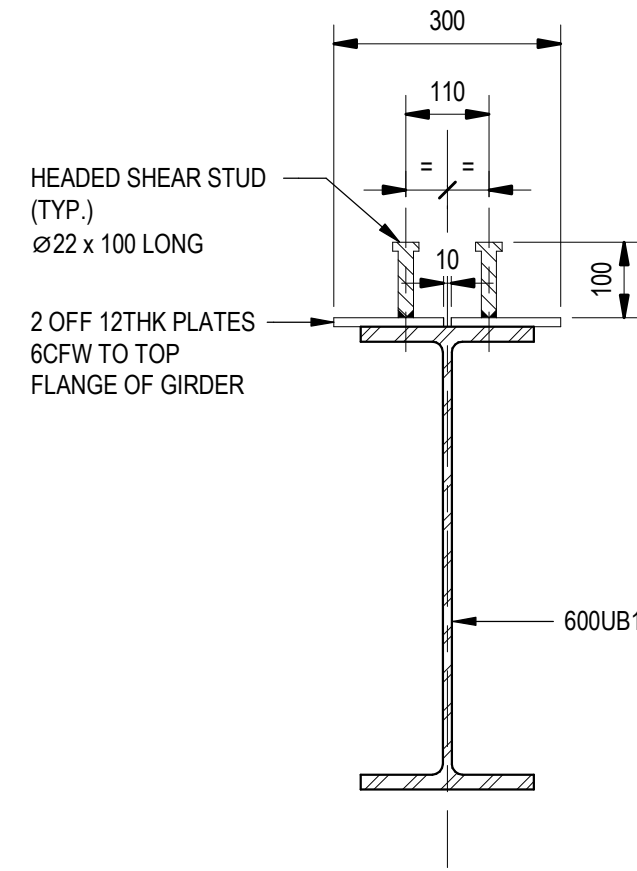
NOTES:

1. FOR GENERAL NOTES REFER DRG 00010.
2. HEADED SHEAR STUDS SHALL BE MANUFACTURED AND INSTALLED IN ACCORDANCE WITH REQUIREMENTS OF AS 1554 PART-2 ARC STUD WELDING (STEEL STUDS TO STEEL) AND THE SPECIFICATION.
3. SHEAR STUDS SHALL HAVE MINIMUM ULTIMATE TENSILE STRENGTH OF 410MPa, AS PER AS 1554.2.
4. TEMPORARY WORKS INCLUDING ERECTION OF STEEL BEAM ARE NOT INCLUDED IN THE DESIGN PACKAGE. CONTRACTOR TO MAKE ALLOWANCE FOR LIFTING, INCLUDING FABRICATION OF LIFTING LUGS OR SIMILAR, DESIGNATION OF LIFTING POINTS AND LIFTING LOAD. LIFTING SCHEME IS TO BE VERIFIED BY THE TEMPORARY WORKS DESIGNER AND AGREED WITH THE SUPERINTEND.



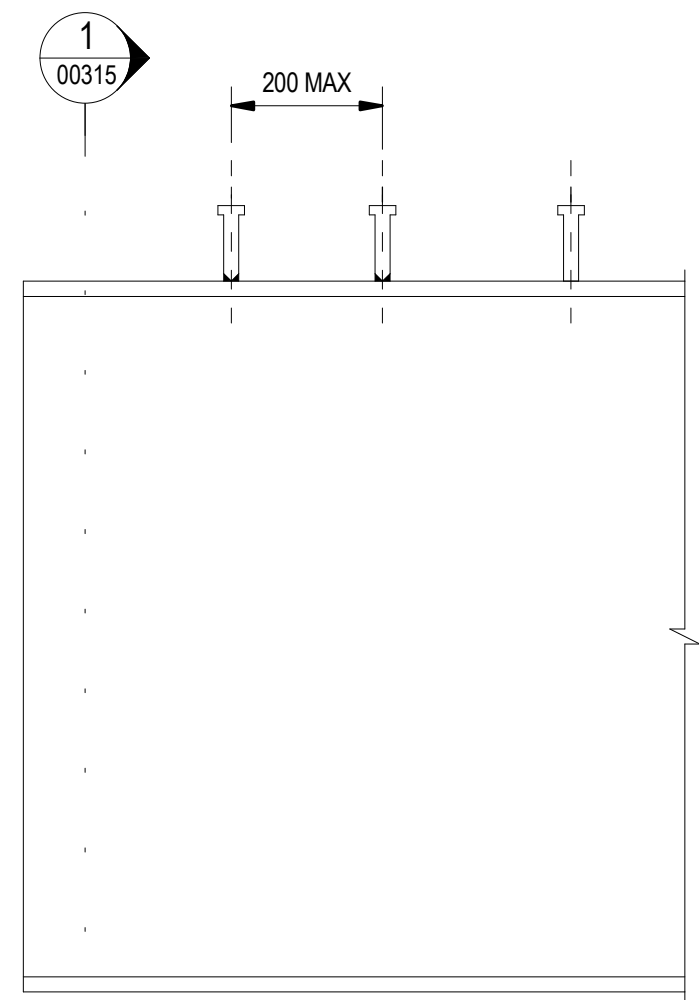
A DETAIL

SCALE 1:10



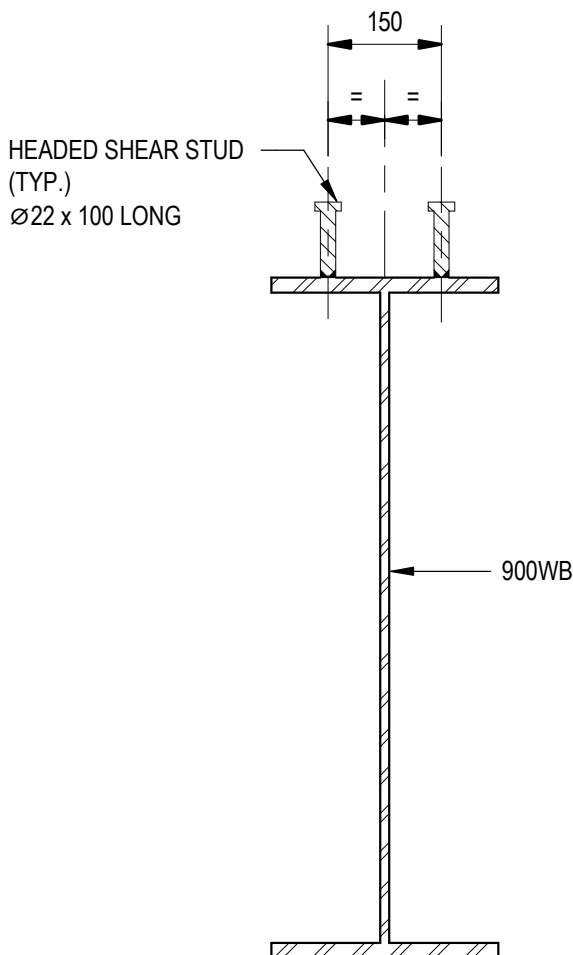
1 SECTION

SCALE 1:10



C SECTION

SCALE 1:10

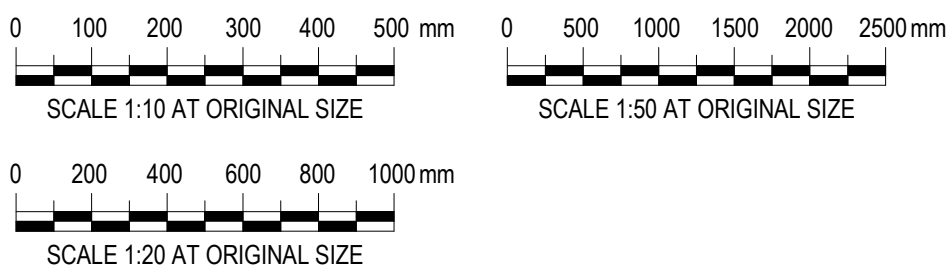


2 SECTION

SCALE 1:10

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Rev	Description	Checked	Approved	Date
P01	90% FINAL DESIGN	AR	LR	07.06.24
Author	A. PERNA	Drafting Check	L. PARRY	
Designer	C. NGUYEN	Design Check	A. RITCHIE	



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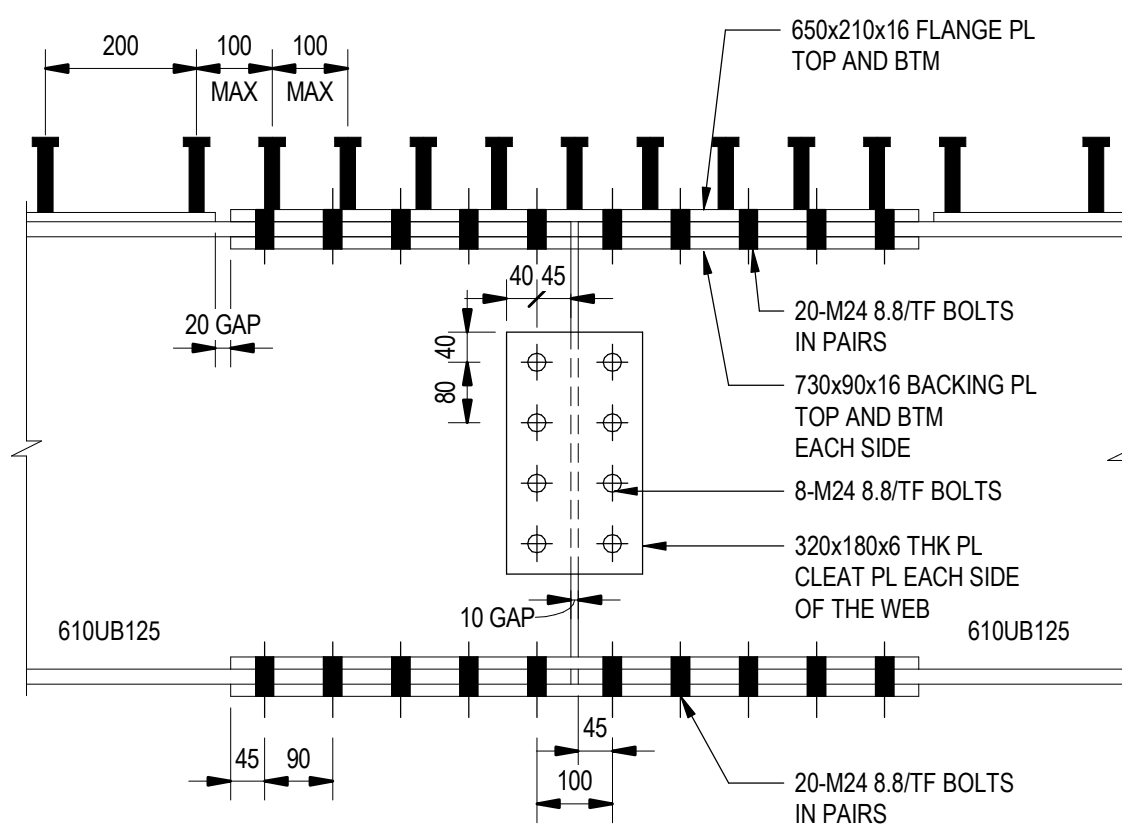
Client	ADELAIDE HILLS COUNCIL
Project	AMY GILLETT BIKEWAY EXTENSION
Status	PRELIMINARY

Drawing Title
TYPICAL STEEL GIRDER DETAILS - SHEET 1

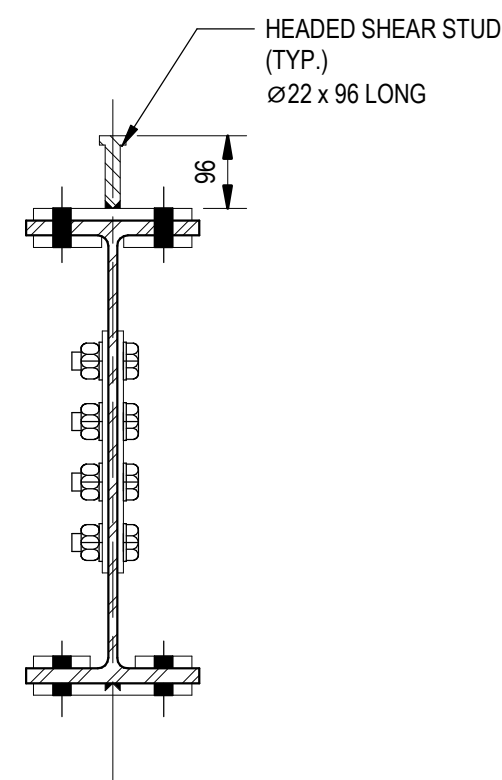
Drawing No.
12638597-GHD-00-00-DRG-ST-00315

Size
A1

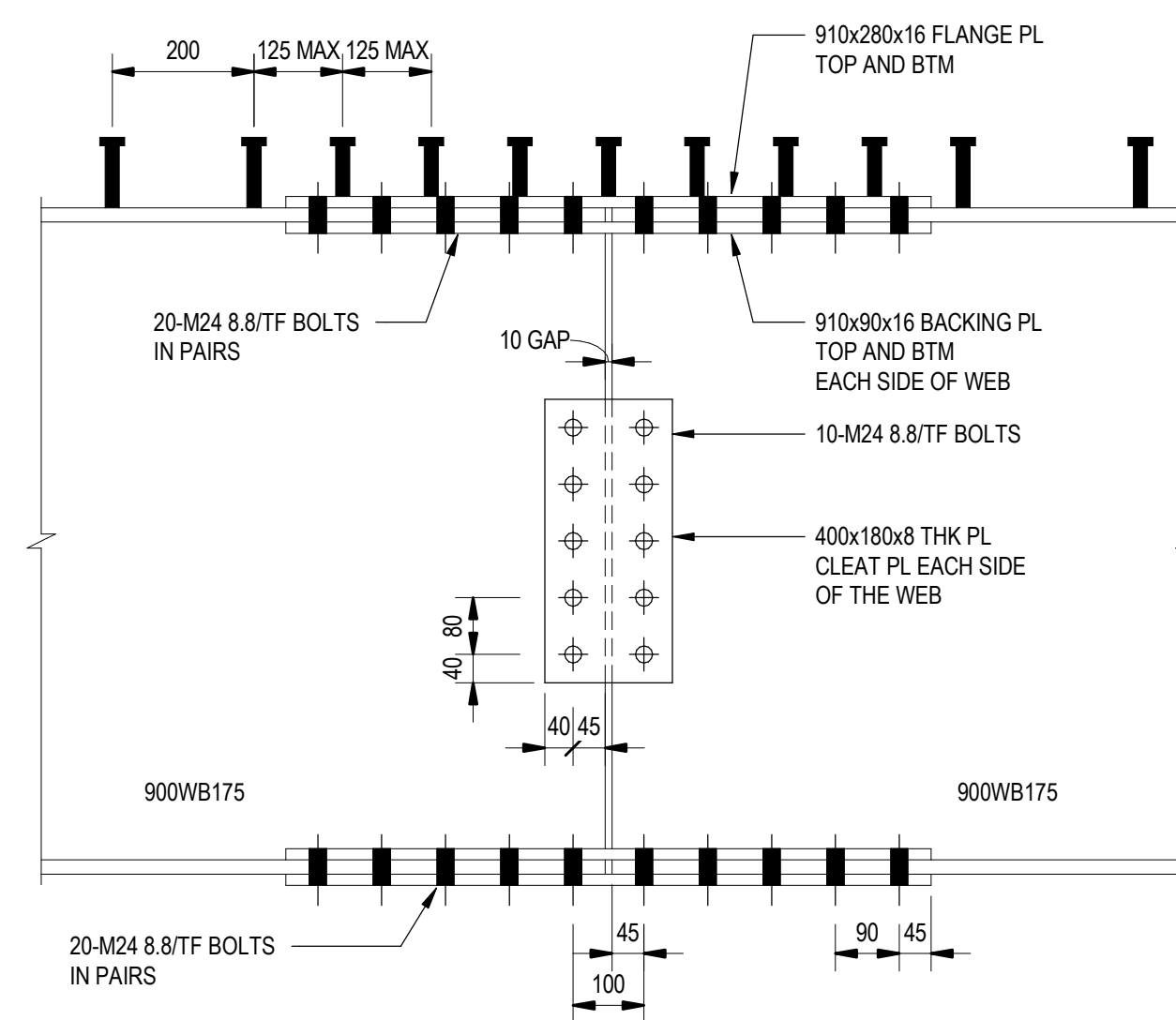
Rev
P01



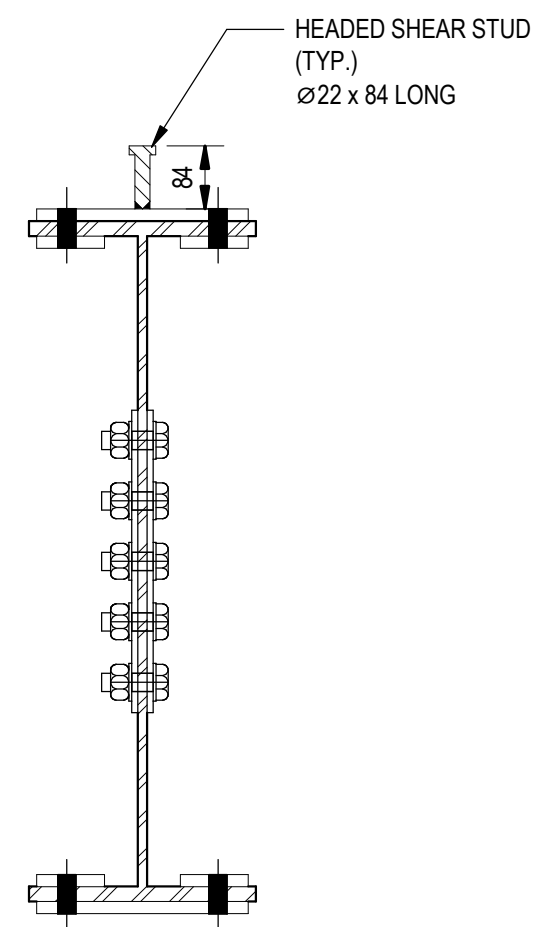
B DETAIL
00315 SCALE 1 : 10



1 SECTION
SCALE 1 : 10



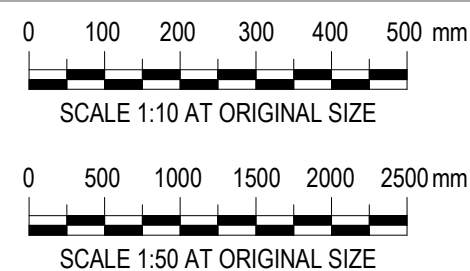
D DETAIL
00315 SCALE 1 : 10



4 SECTION
SCALE 1 : 10

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P01	90% FINAL DESIGN	AR	LR	07.06.24
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Author	A. PERNA	Drafting Check	L. PARRY	
Designer	C. NGUYEN	Design Check	A. RITCHIE	



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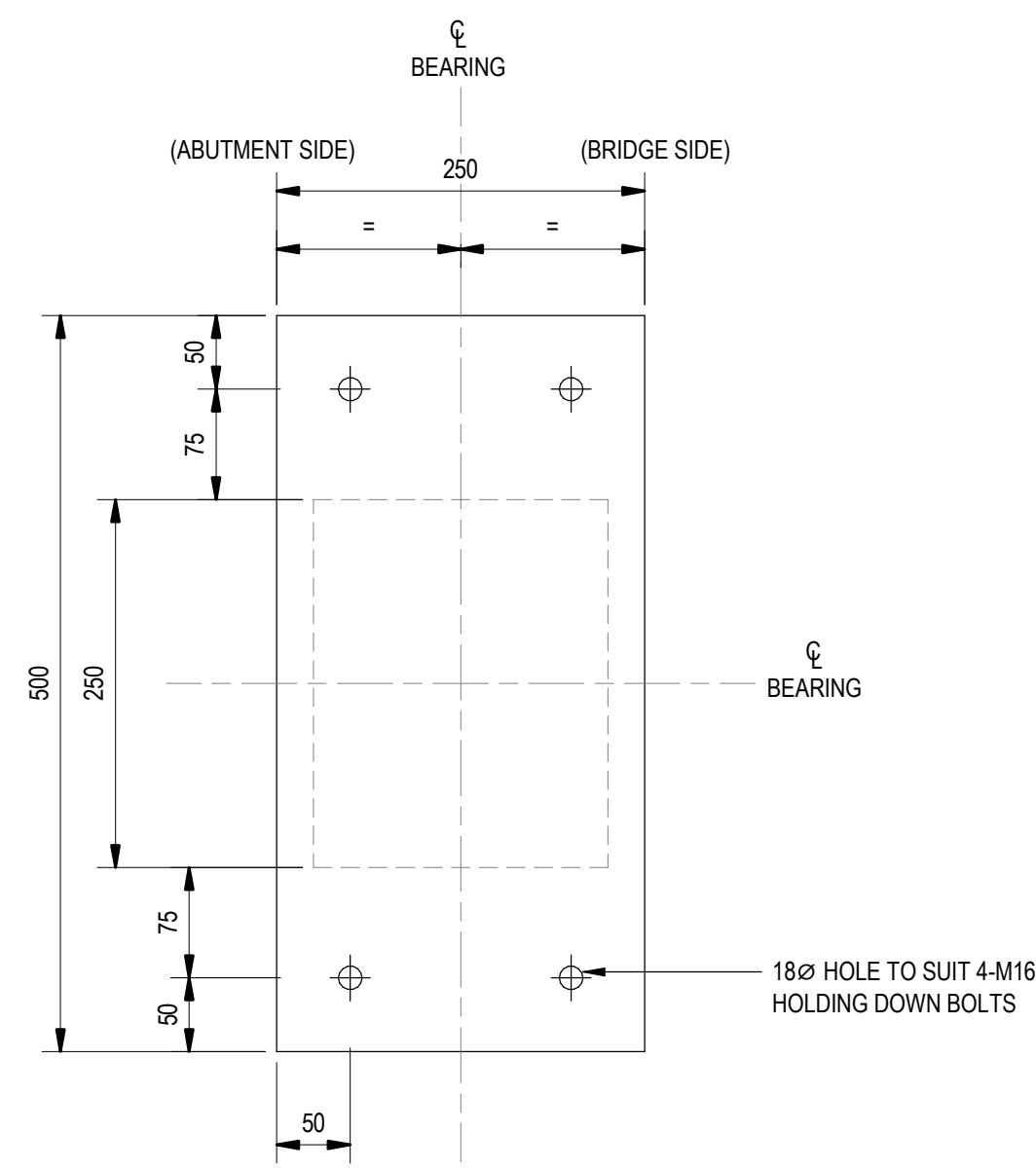
Client **ADELAIDE HILLS COUNCIL**
Project **AMY GILLETT BIKEWAY EXTENSION**
Status **PRELIMINARY**

Drawing Title
TYPICAL STEEL GIRDER DETAILS - SHEET 2

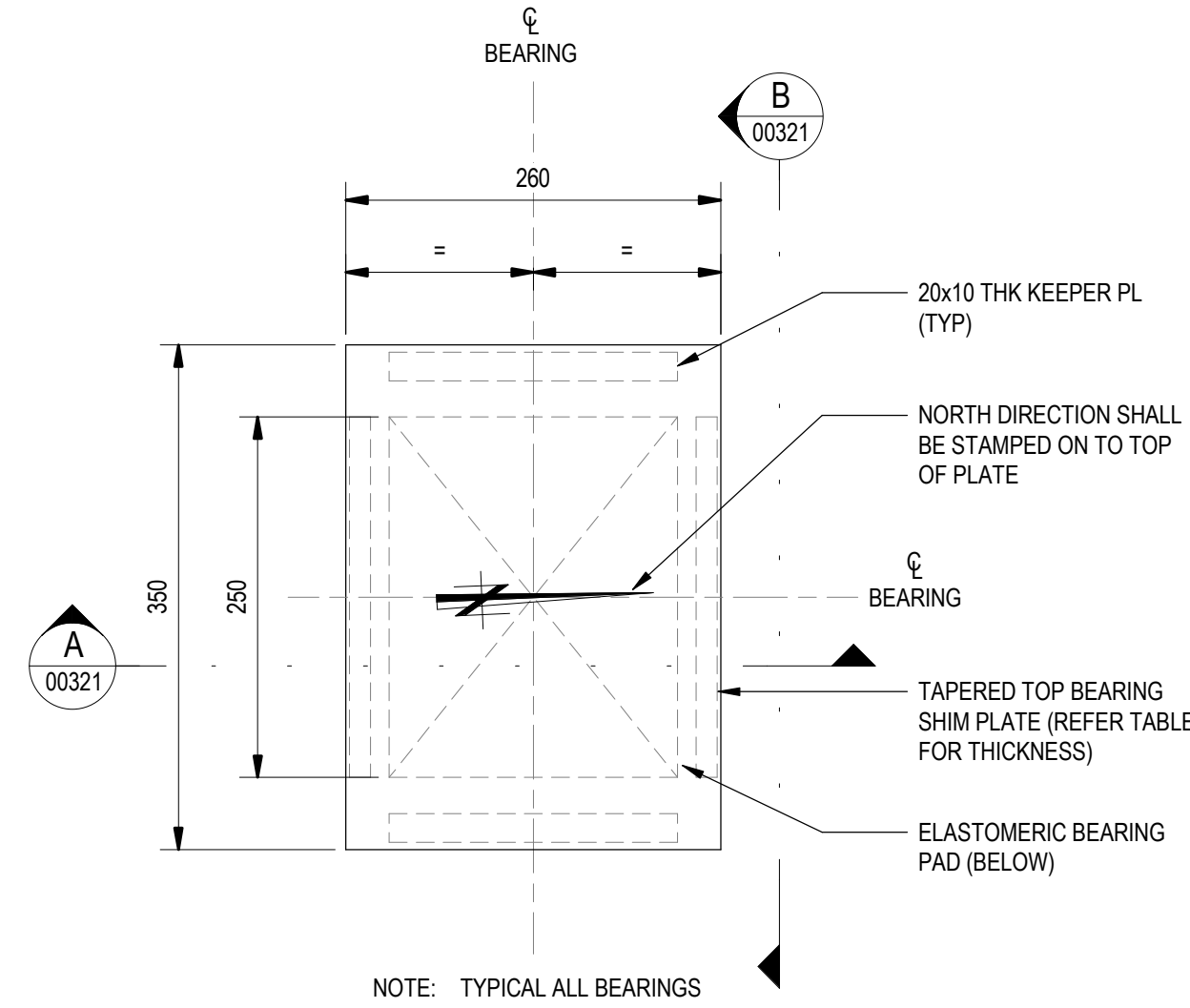
Drawing No.
12638597-GHD-00-00-DRG-ST-00316

Size
A1

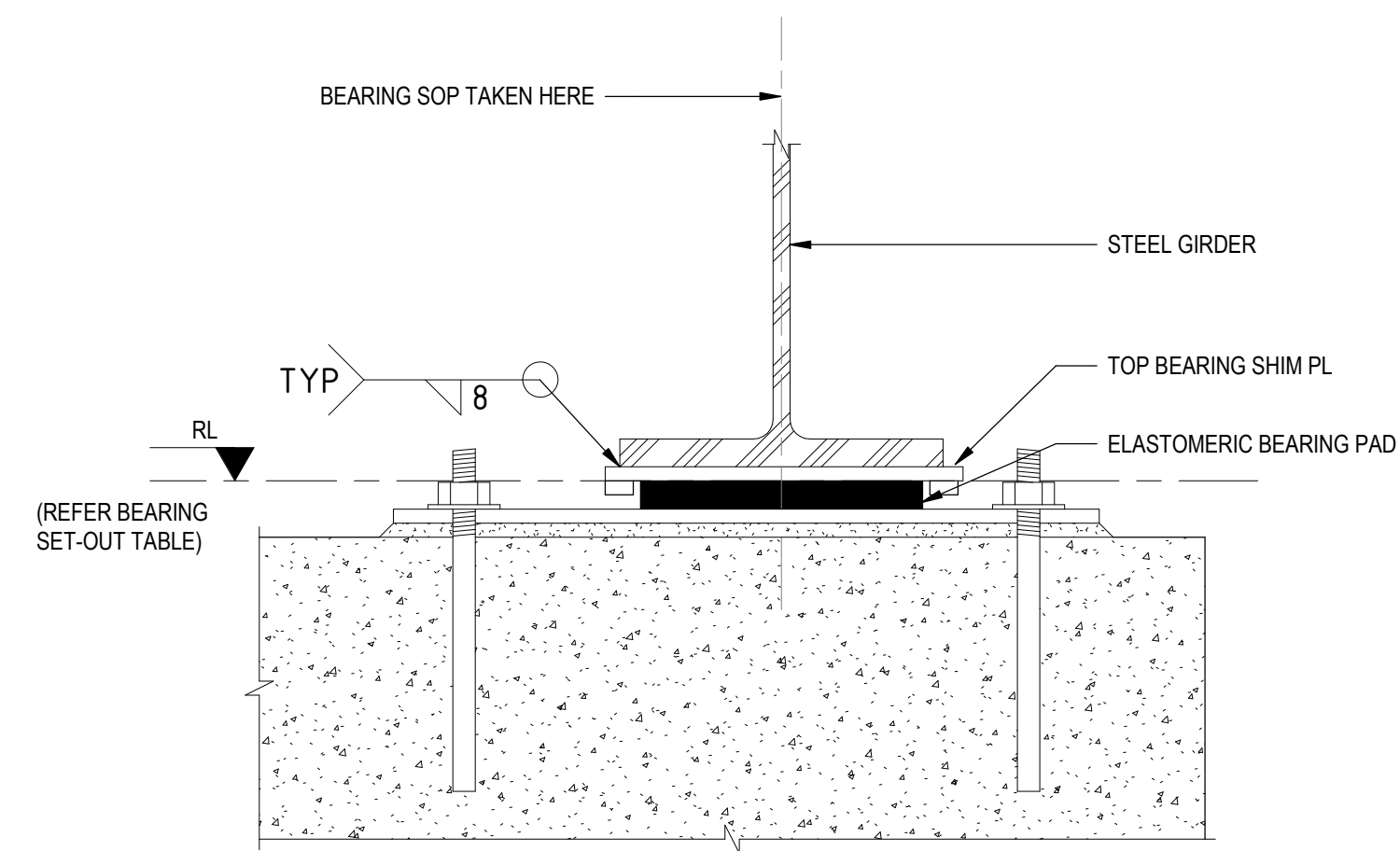
Rev
P01



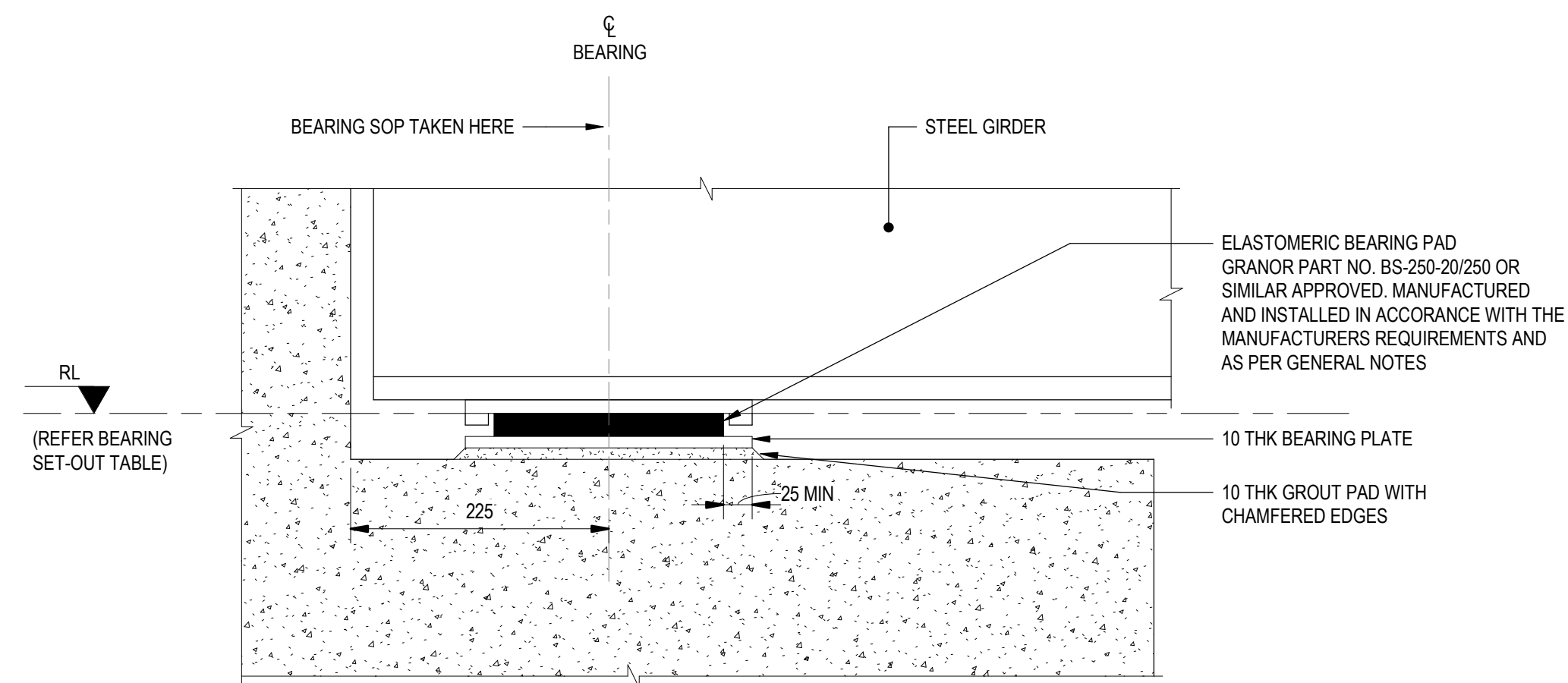
BEARING BASE PLATE DETAIL
SCALE 1:5



BEARING TOP PLATE DETAIL
SCALE 1:5



B SECTION
SCALE 1:5

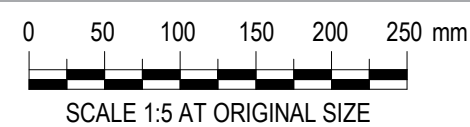


NOTE: VIEW SHOWING BEARING ORIENTATION IMMEDIATELY PRIOR TO LOADING

A SECTION
SCALE 1:5

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P01	90% FINAL DESIGN	AR	LR	07.06.24
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Project No.
12638597

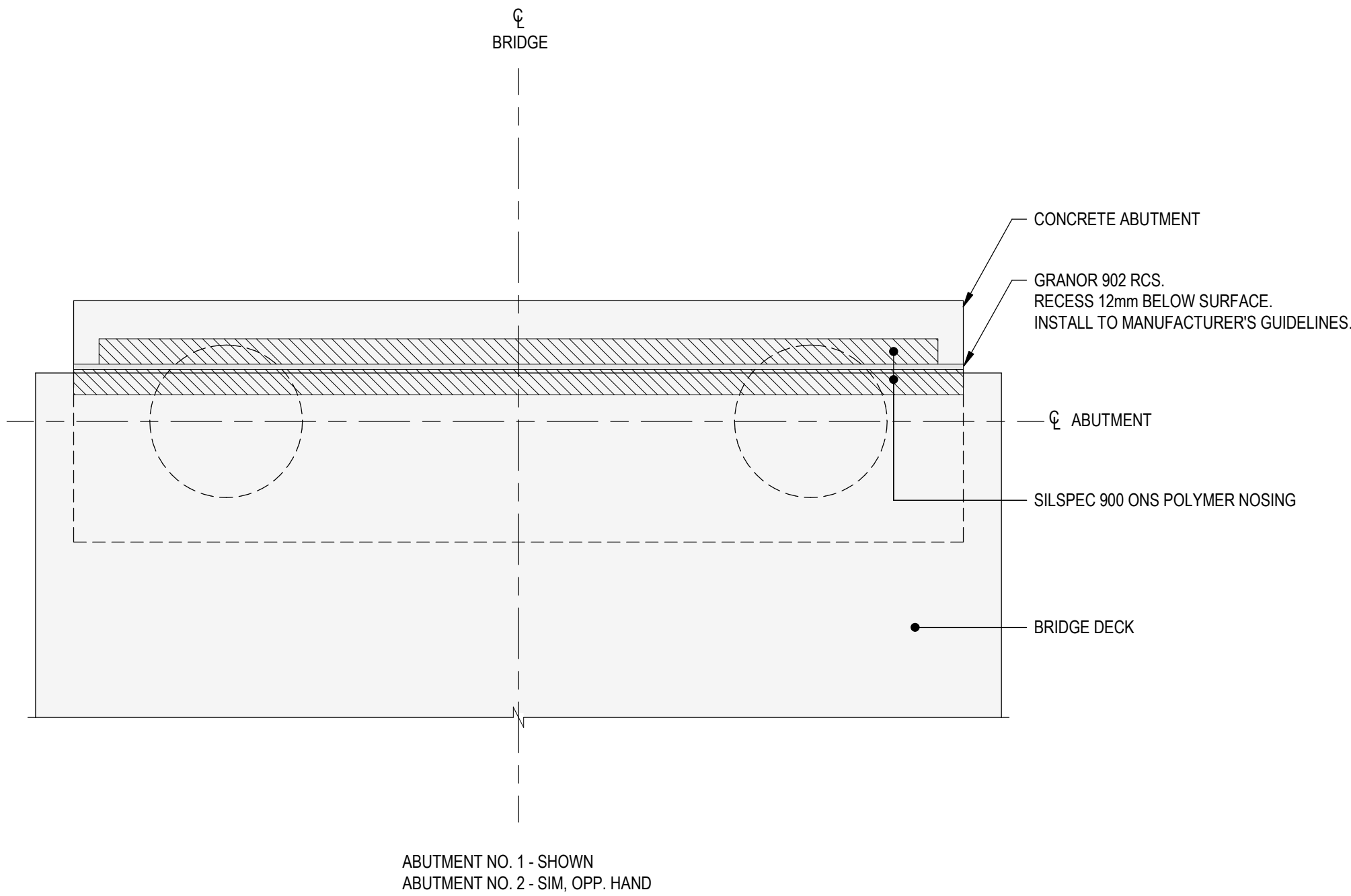
Client	ADELAIDE HILLS COUNCIL
Project	AMY GILLETT BIKEWAY EXTENSION
Status	PRELIMINARY

Drawing Title
TYPICAL BEARING DETAILS

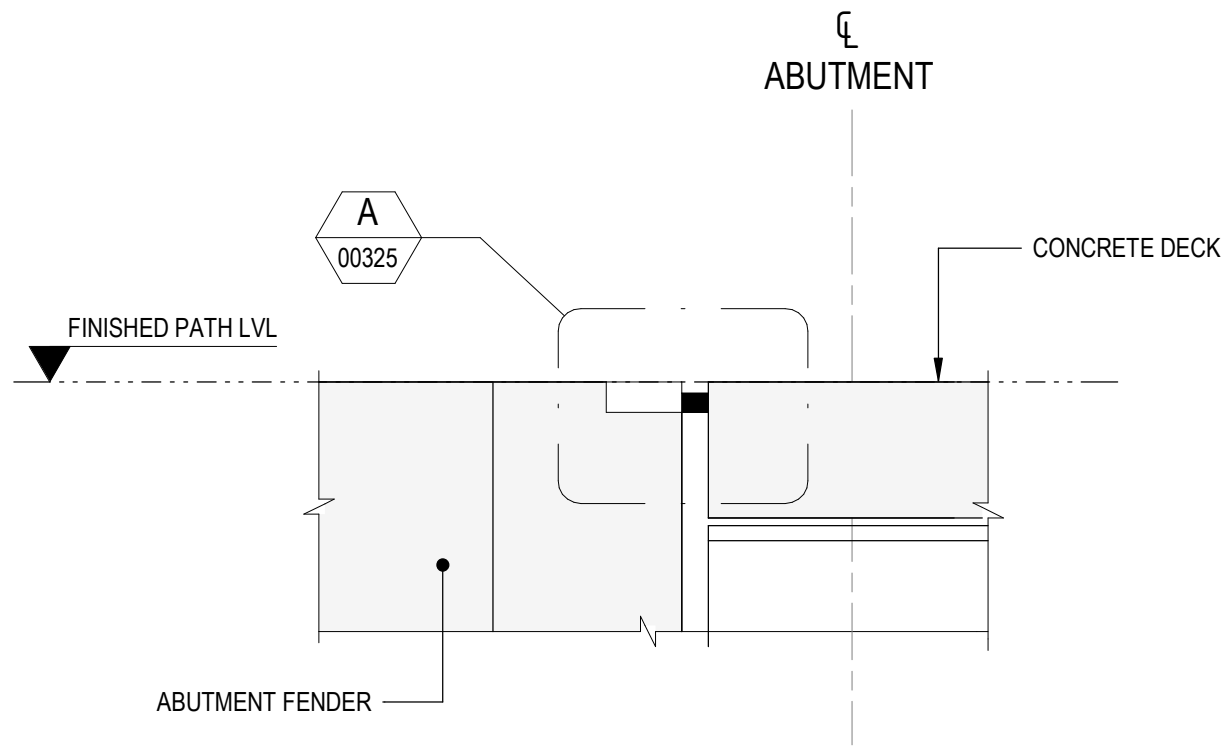
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Size
A1

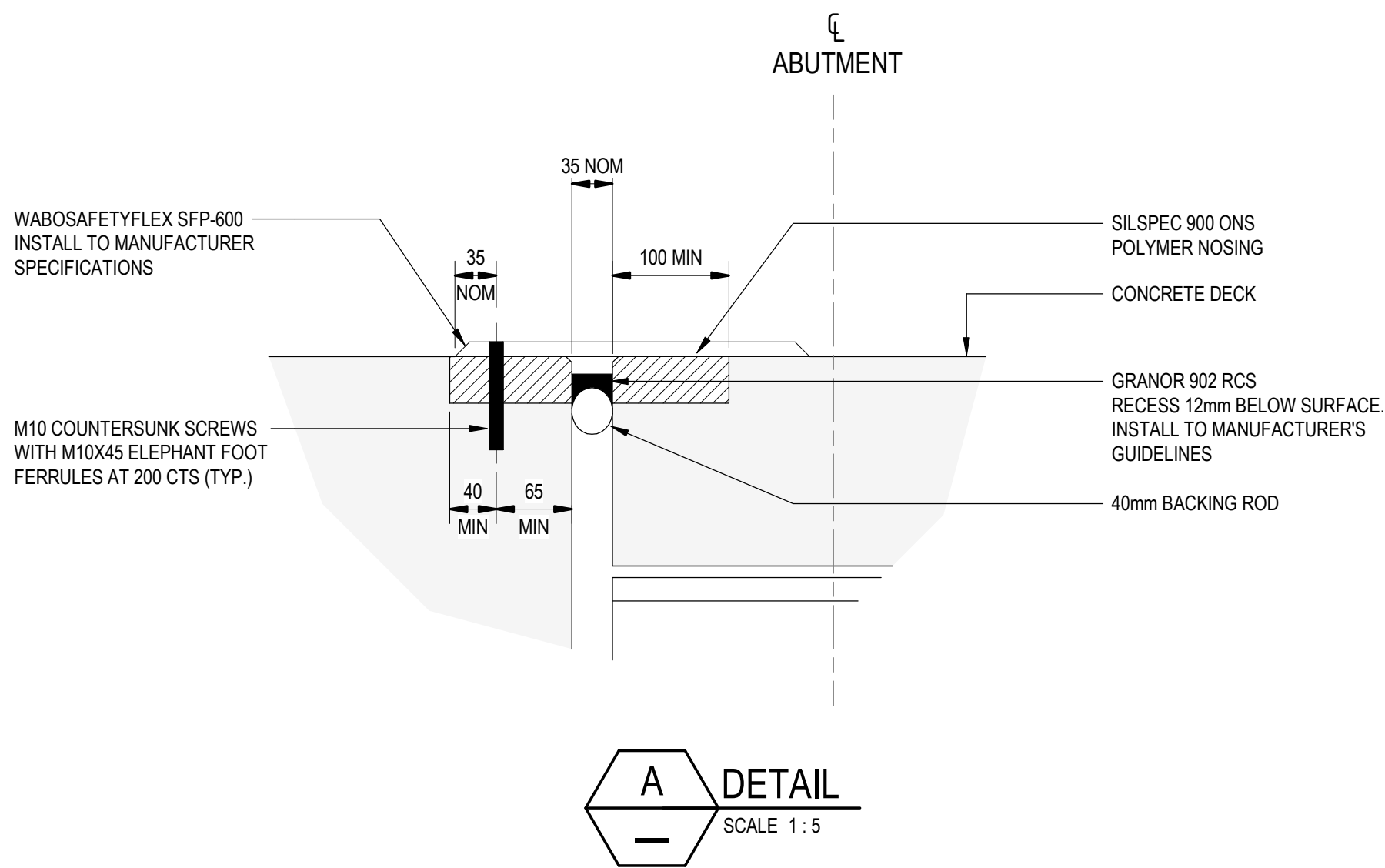
Rev
P01



EXPANSION JOINT - PLAN
SCALE 1 : 20



EXPANSION JOINT INSTALLATION DETAIL
SCALE 1 : 10



A DETAIL
SCALE 1 : 5

- NOTES:**
- FOR GENERAL NOTES REFER TO DRAWING No. 00010 AND 00011.
 - EXPANSION JOINT SHALL BE GRANOR 902 RCS XJS EXPANSION JOINT SYSTEM OR SIMILAR APPROVED. ALL COMPONENTS AND INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS.
 - THE INSTALLATION GAP AT 20°C. GAP SHALL BE INCREASED AND DECREASED BY 1.9mm FOR EACH 5°C FALL / RISE IN TEMPERATURE. BACKWALL THICKNESS MAY BE ADJUSTED BY UP TO 10mm TO ACHIEVE THIS.

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P01	90% FINAL DESIGN	AR	LR	07.06.24
Rev	Description	Checked	Approved	Date
Author	A. PERNA	Drafting Check	L. PARRY	
Designer	C. NGUYEN	Design Check	A. RITCHIE	



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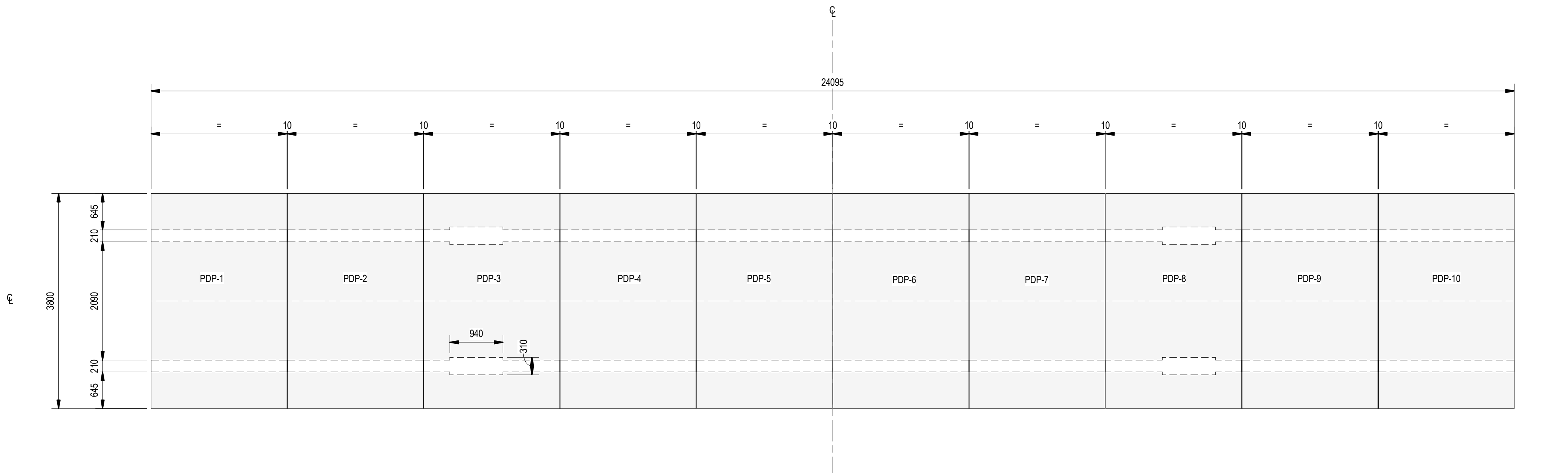
Client	ADELAIDE HILLS COUNCIL
Project	AMY GILLETT BIKEWAY EXTENSION
Status	PRELIMINARY

Drawing Title **TYPICAL EXPANSION JOINT DETAILS**

Drawing No.
12638597-GHD-00-00-DRG-ST-00325

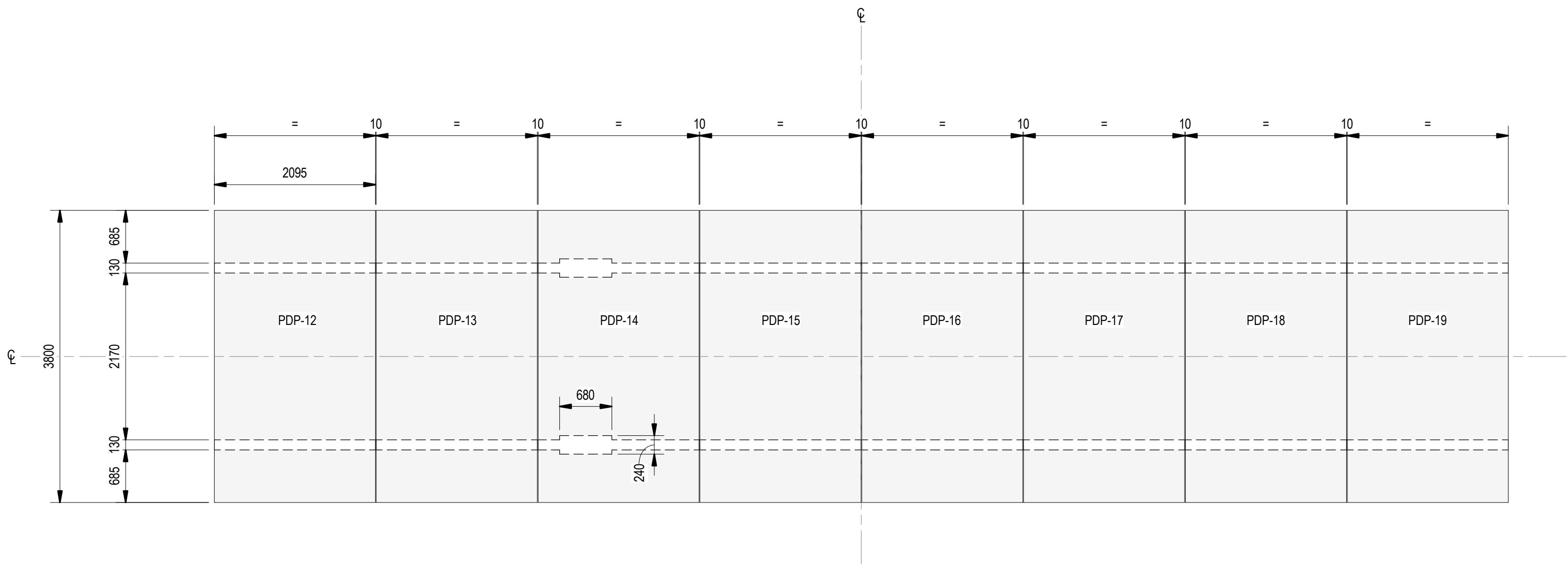
Size
A1

Rev
P01



PRECAST DECK PANEL LAYOUT - ANGAS CREEK

SCALE 1 : 50



PRECAST DECK PANEL LAYOUT - WILLIAM CREEK

SCALE 1 : 50

NOTES:

1. FOR GENERAL NOTES REFER TO DRG. 00010 AND 00011.
2. PRECAST DECK PANELS HAVE BEEN DESIGNED FOR THE SELFWEIGHT OF THE CAST IN-SITU DECK AND 1KPa CONSTRUCTION LIVE LOAD.

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Project No.
12638597

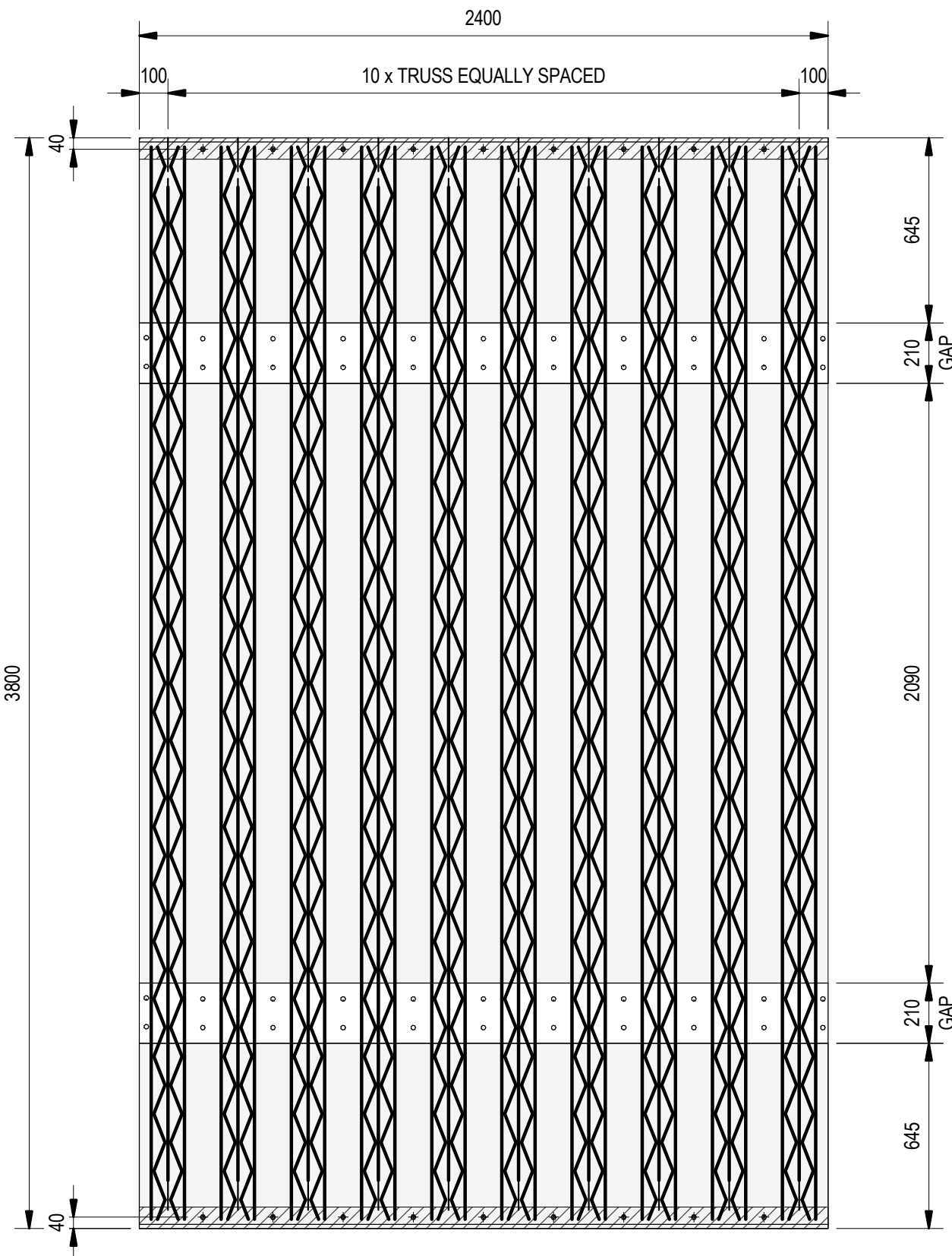
Client	ADELAIDE HILLS COUNCIL
Project	AMY GILLETT BIKEWAY EXTENSION
Status	PRELIMINARY

Drawing Title
TYPICAL PRECAST DECK PANEL LAYOUT

Drawing No.
12638597-GHD-00-00-DRG-ST-00331

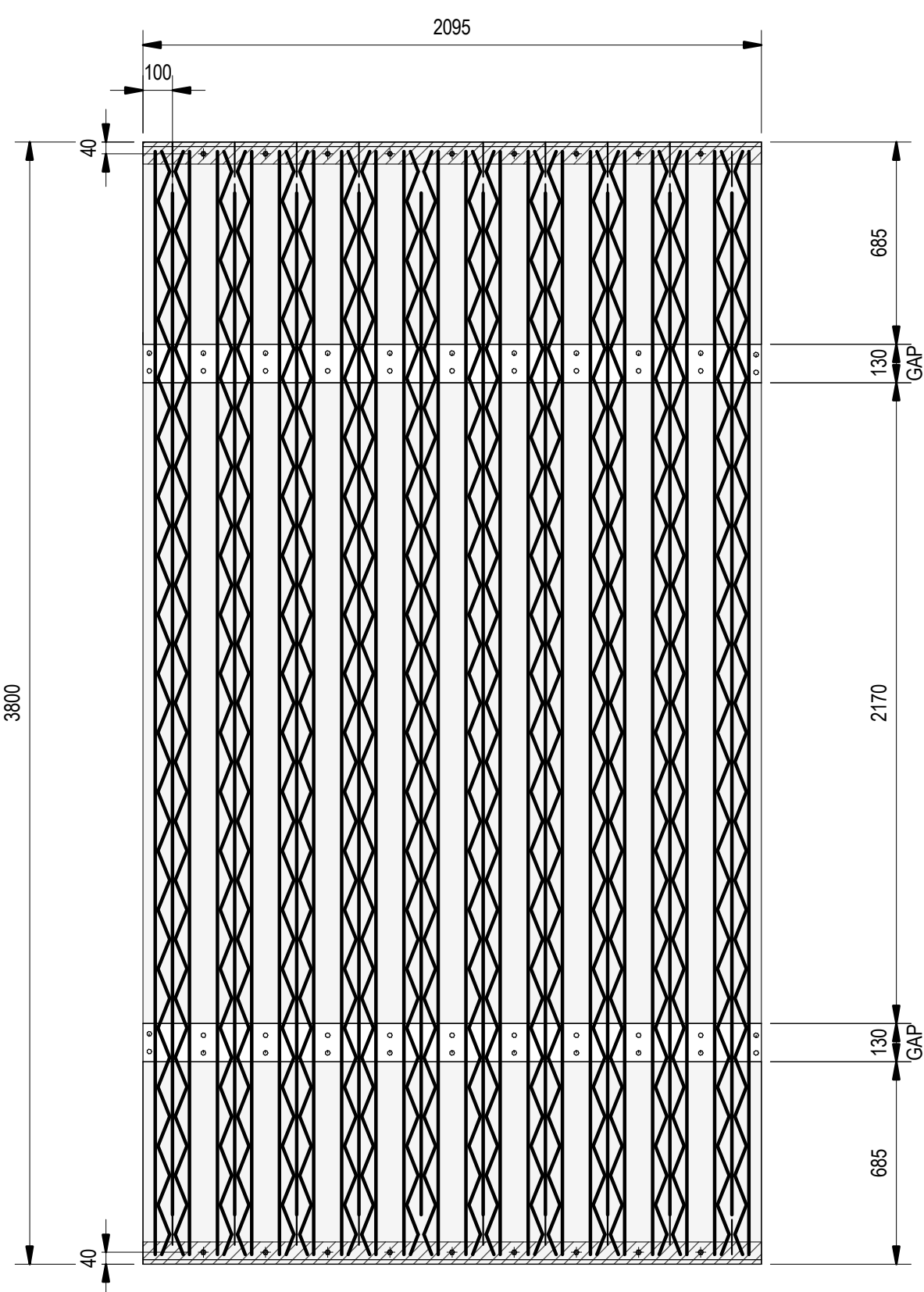
Size
A1

Rev
P01



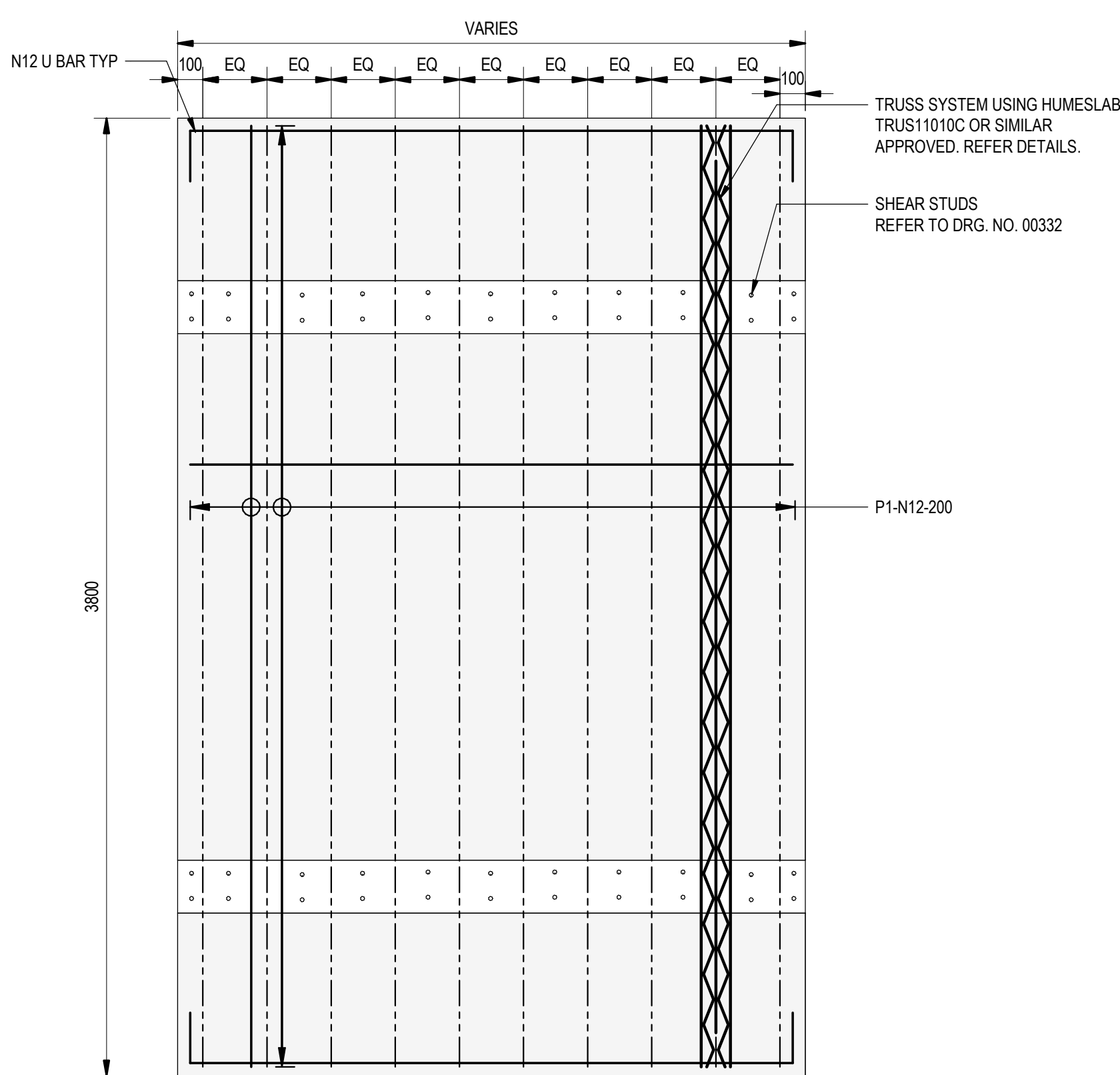
TRUSS LAYOUT - PRECAST PANEL PDP-1TP PDP-10

SCALE 1 : 20



TRUSS LAYOUT - PRECAST PANEL PDP-12 TO PDP-19

SCALE 1 : 20

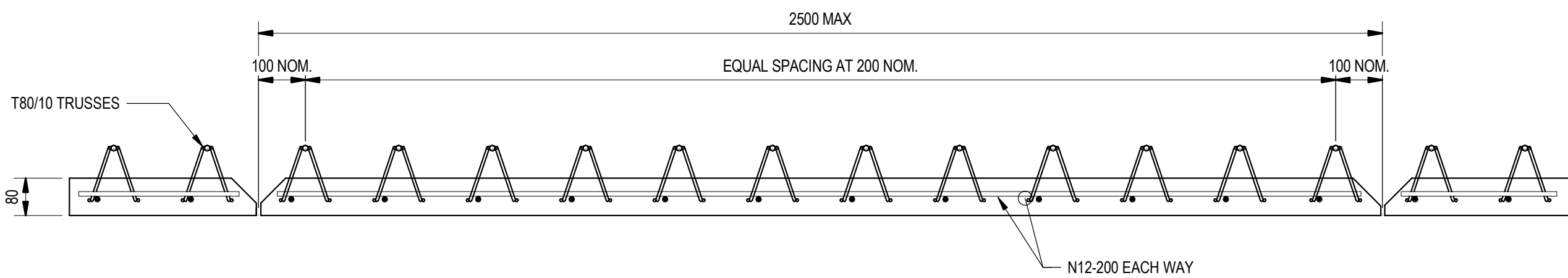


PRECAST PLAN TYPICAL REINFORCEMENT

SCALE 1 : 20

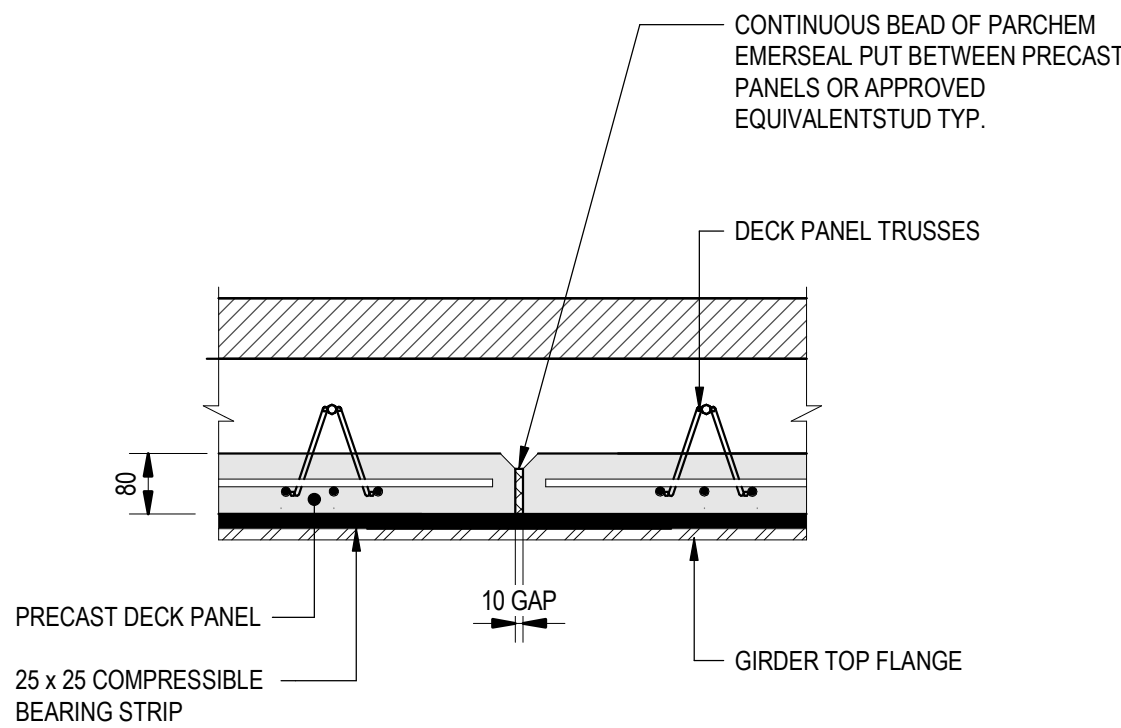
NOTES:

1. FOR GENERAL NOTES REFER TO DRG. 00010 AND 00011.
2. PRECAST DECK PANELS HAVE BEEN DESIGNED FOR THE SELFWEIGHT OF THE CAST IN-SITU DECK AND 1kPa CONSTRUCTION LIVE LOAD.



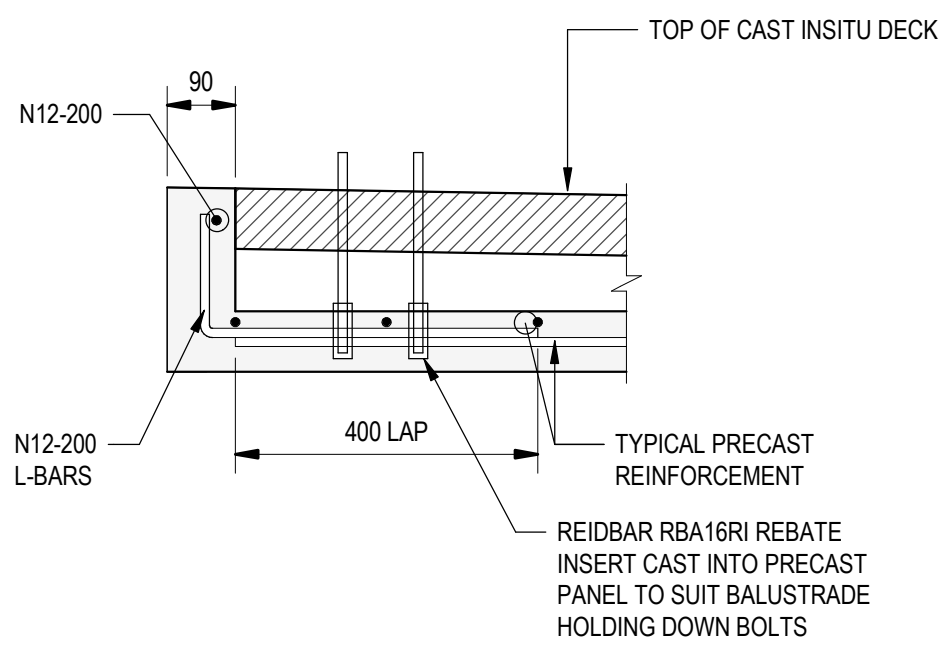
TYPICAL REINFORCEMENT SECTION PRECAST DECK PANEL

SCALE 1 : 10



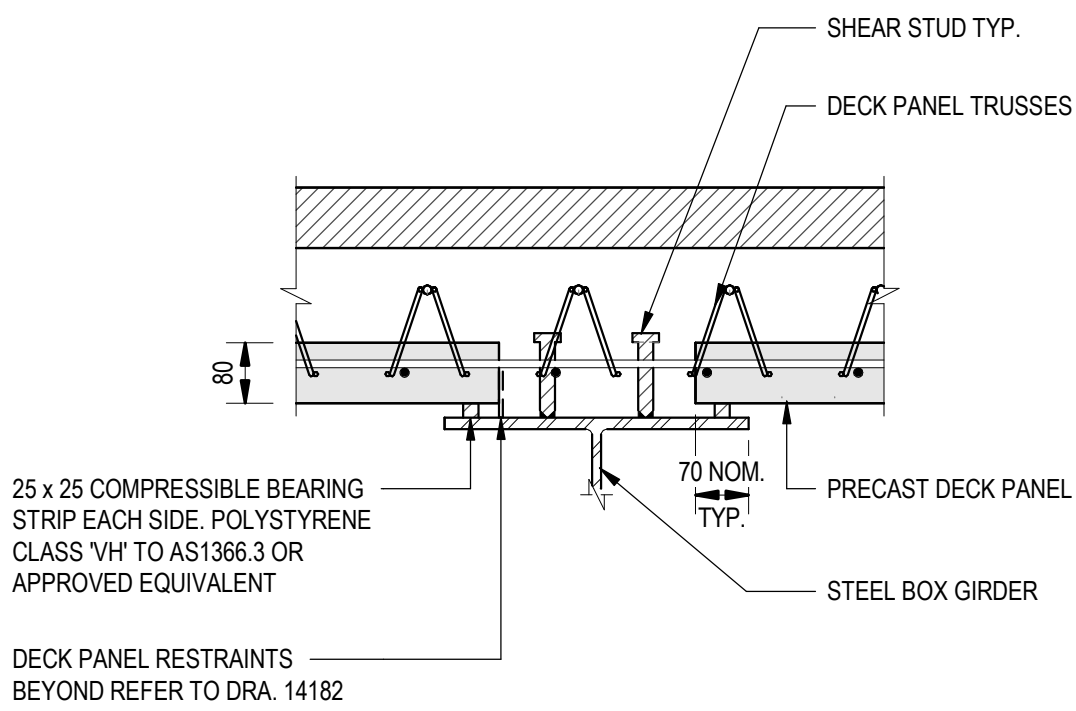
TYPICAL PRECAST DECK PANEL JOINT DETAIL

SCALE 1 : 10



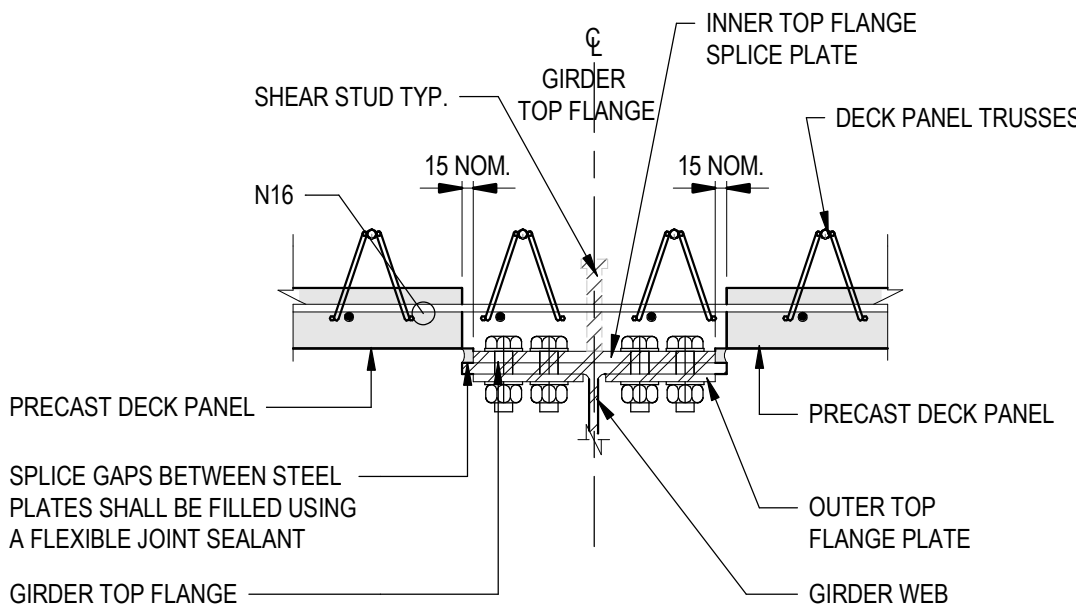
TYPICAL PRECAST DECK PANEL UPSTAND DETAIL

SCALE 1 : 10



PRECAST DECK PANEL DETAIL AT GIRDER FLANGE

SCALE 1 : 10



TYPICAL PRECAST DECK PANEL AT SPLICE LOCATION

SCALE 1 : 10

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P01	90% FINAL DESIGN	AR	LR	07.06.24
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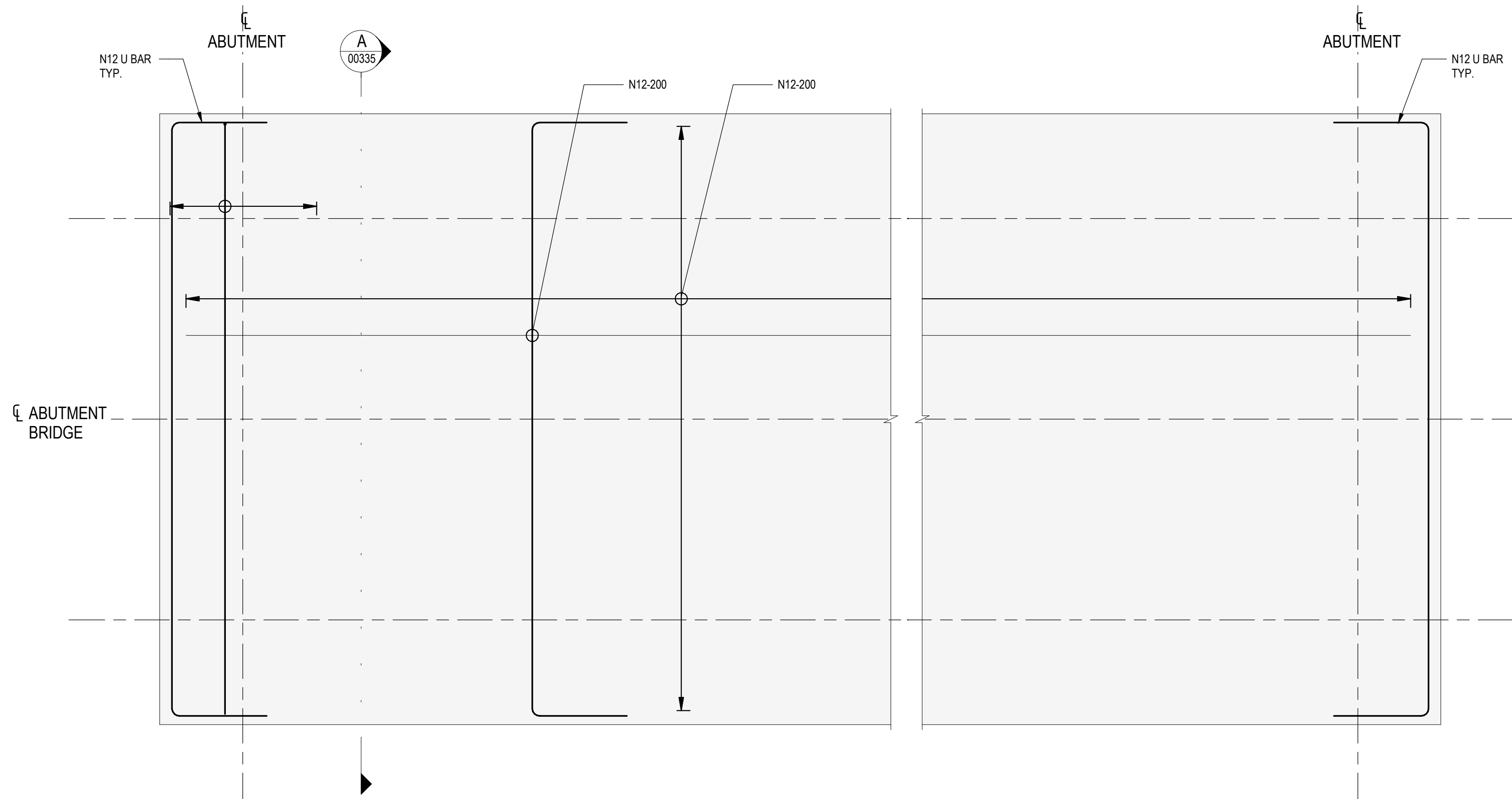
Project No.
12638597

Client **ADELAIDE HILLS COUNCIL**
Project **AMY GILLETT BIKEWAY EXTENSION**
Status **PRELIMINARY**

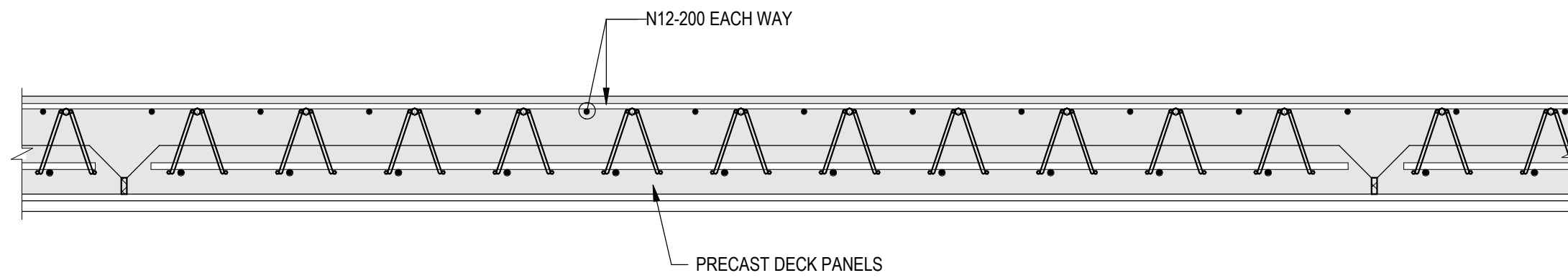
Drawing Title
TYPICAL PRECAST DECK PANEL DETAILS

Drawing No.
12638597-GHD-00-00-DRG-ST-00332

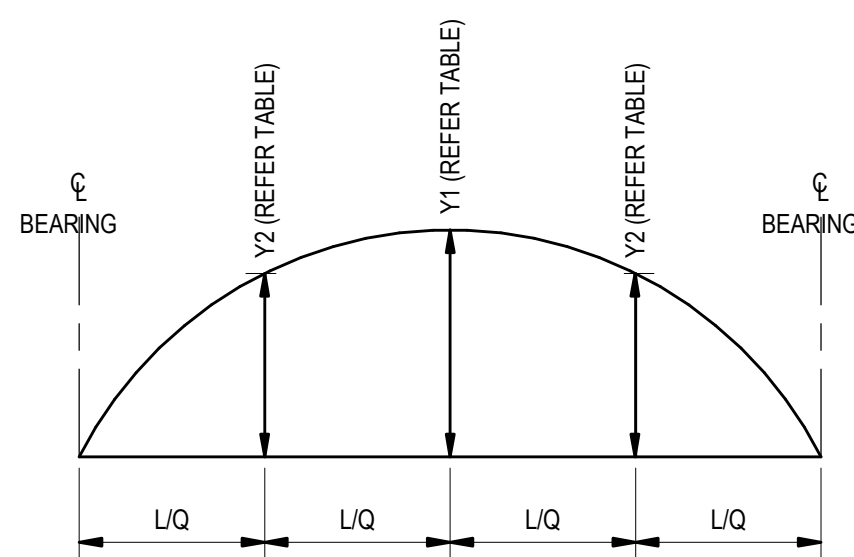
Size
A1
Rev
P01



DECK REINFORCEMENT PLAN
SCALE 1:20



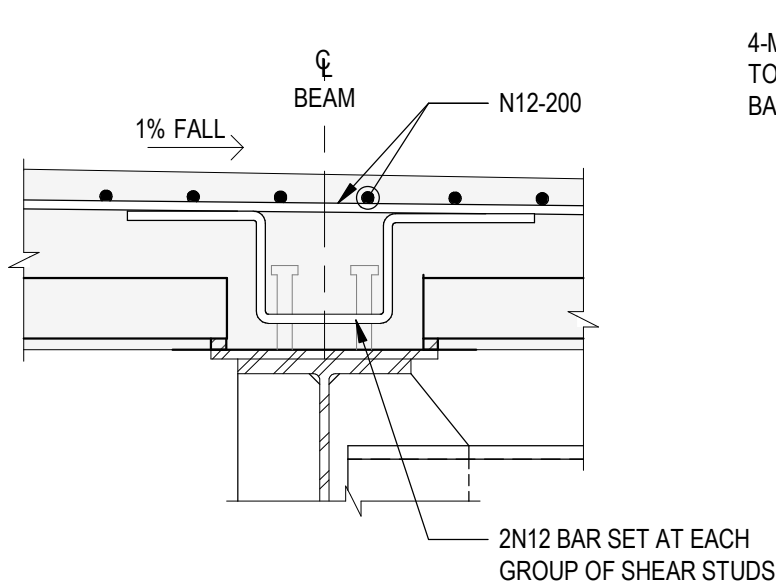
TYPICAL REINFORCEMENT SECTION - CAST INSITU DECK
SCALE 1:10



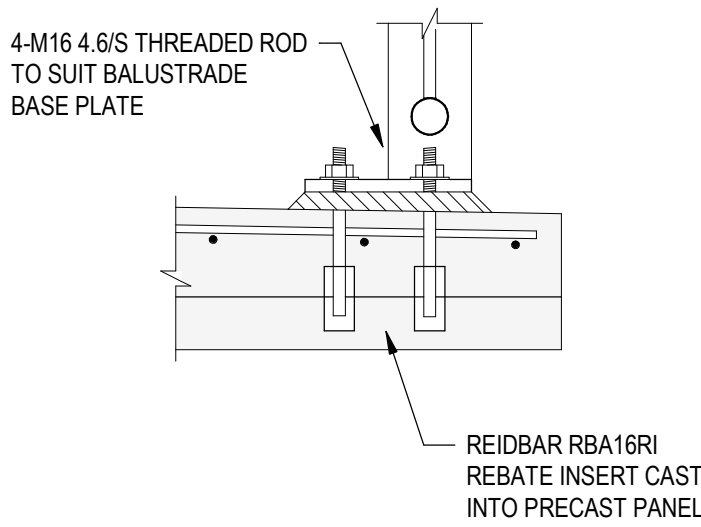
STEEL BOX GIRDER DECK SCREED CAMBER SCHEDULE		
BEAM	Y1 (mm)	Y2 (mm)
600UB125	110	79
900WB175	135	97

NOTE: REQUIRED DECK SCREED ABOVE DESIGN SURFACE LEVEL (EXCLUDING DEFLECTION DUE TO SELF-WEIGHT OF STEEL GIRDERS AND PRECAST PANELS). CAMBER INCLUDES DEFLECTION DUE TO DECK CASTING, PRECAST BARRIERS, AND LONG TERM CREEP & SHRINKAGE

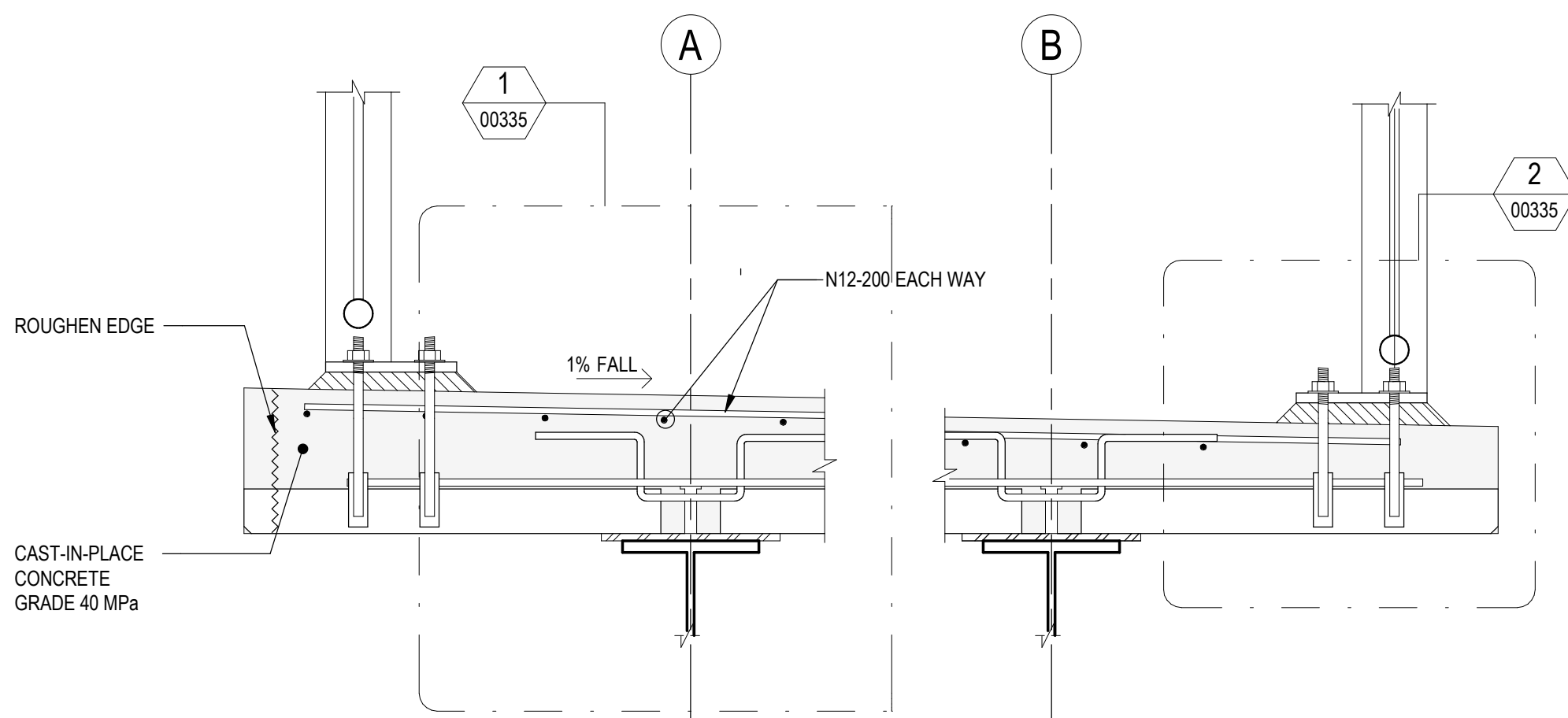
DECK SCREED CAMBER DIAGRAM
NOT TO SCALE



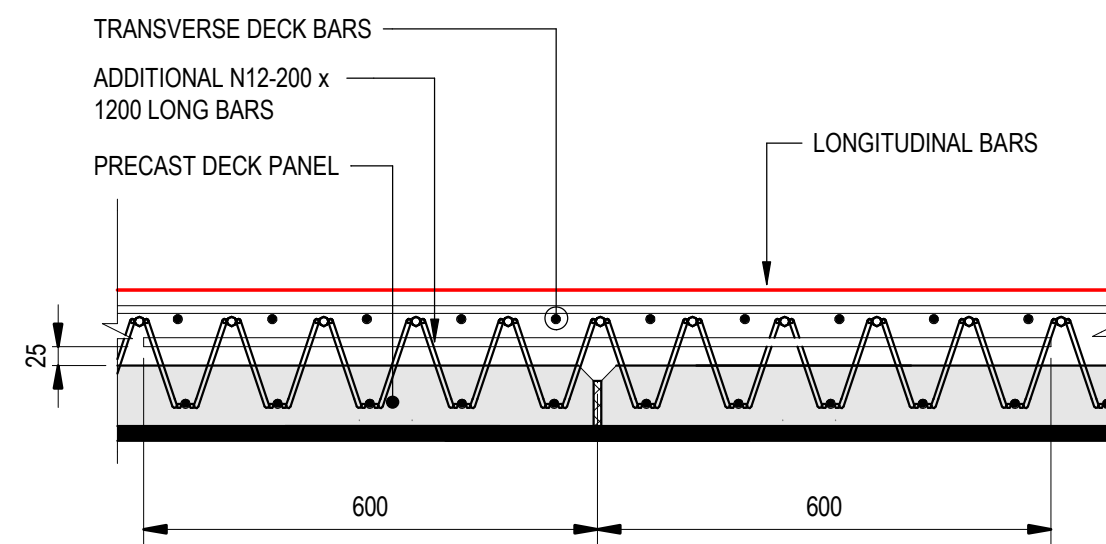
1 DETAIL
00335 SCALE 1:10



2 DETAIL
00335 SCALE 1:10



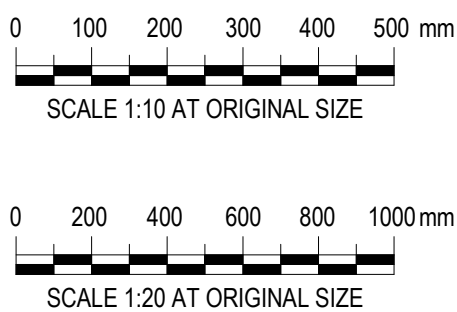
A SECTION
00335 SCALE 1:10



TYPICAL DETAIL BETWEEN PRECAST DECK PANELS
SCALE 1:10

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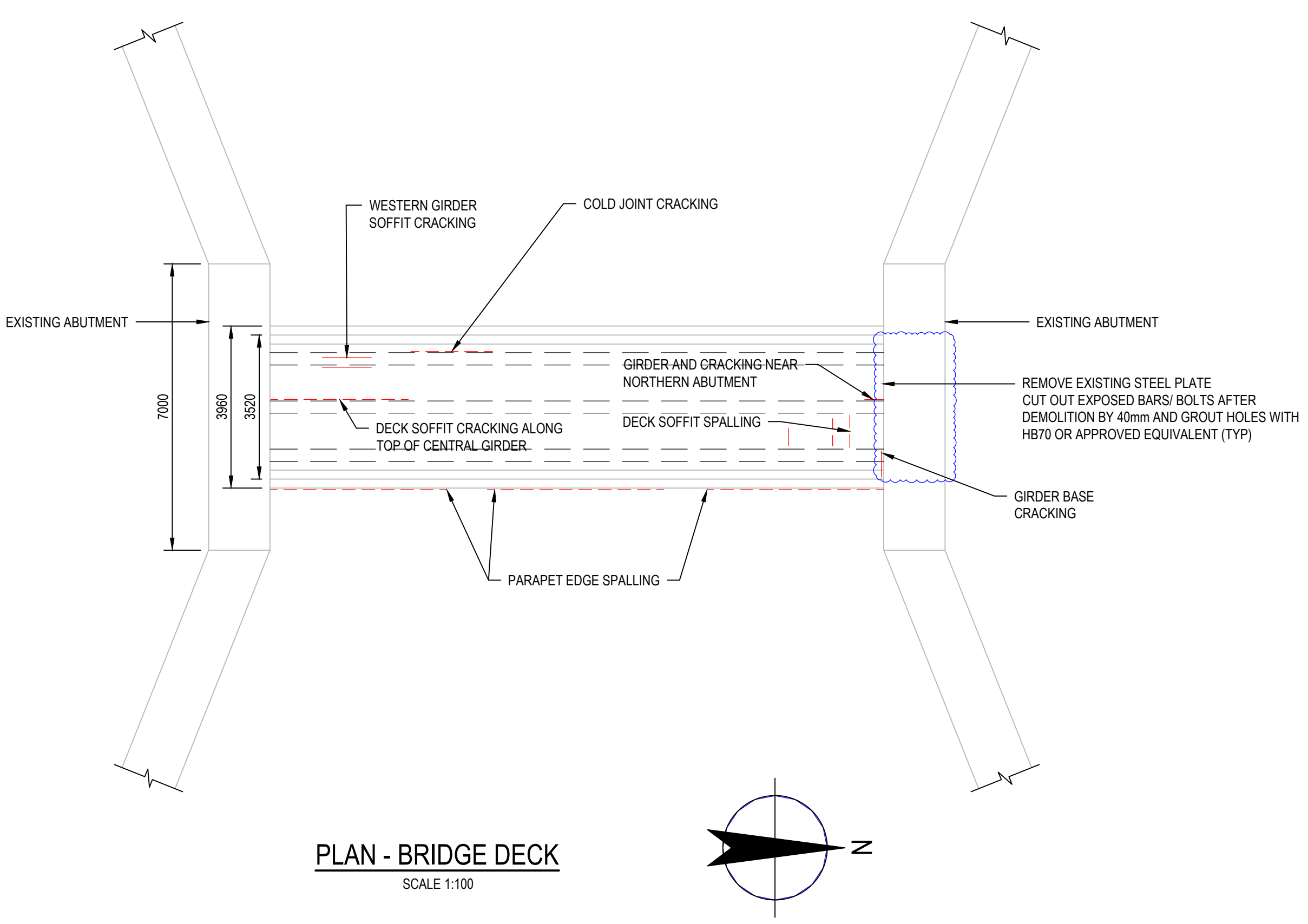
Client **ADELAIDE HILLS COUNCIL**
Project **AMY GILLETT BIKEWAY EXTENSION**
Status **PRELIMINARY**

Drawing Title
**TYPICAL IN SITU DECK
REINFORCEMENT DETAILS**

Drawing No.
12638597-GHD-00-00-DRG-ST-00335

Size
A1

Rev
P01

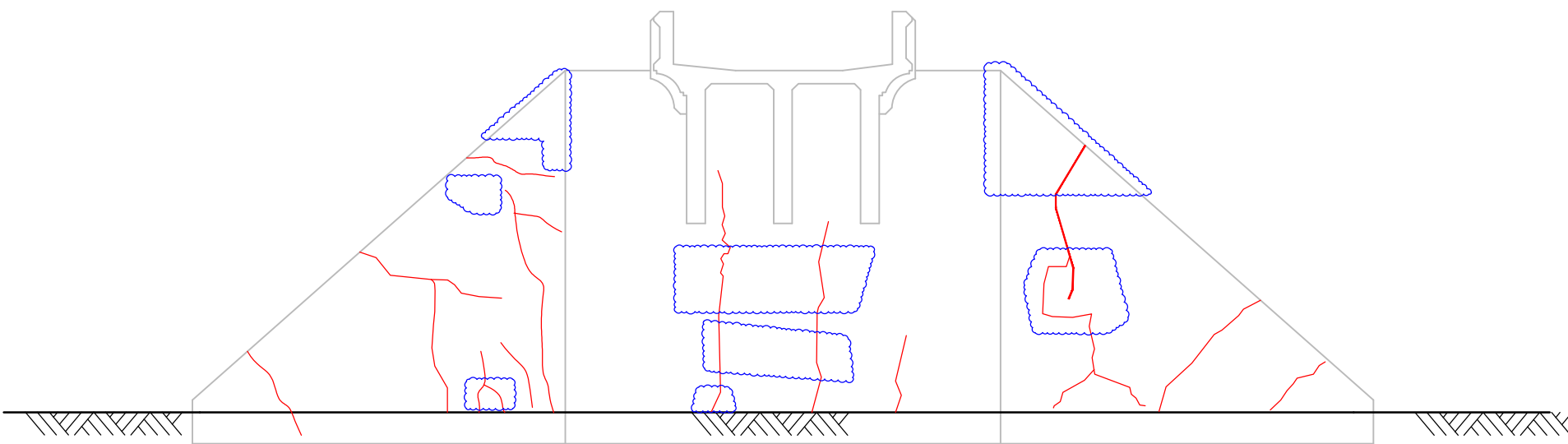


PLAN - BRIDGE DECK
SCALE 1:100

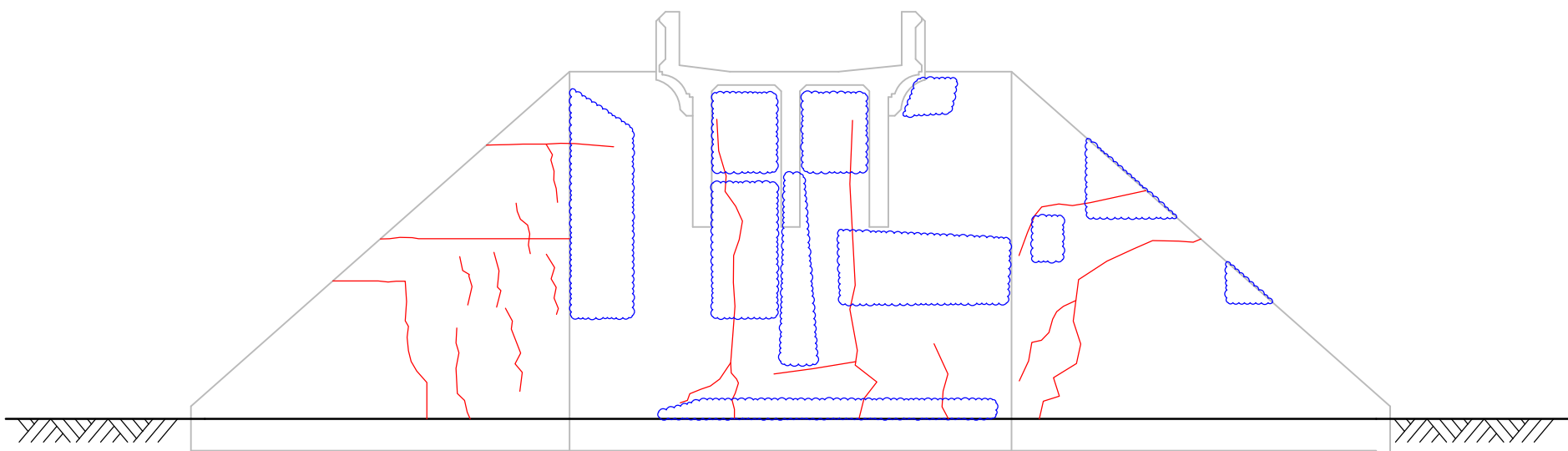
- NOTES:
1. THIS DRAWING TO BE READ IN CONJUNCTION WITH REPAIR SPECIFICATION DOC xxx.
 2. REFER TO SHEET xxxx FOR GENERAL NOTES.

LEGEND

- CRACKING: ADOPT CRACK REPAIR METHODOLOGY OUTLINE IN SECTION 1
- CONCRETE SPALLING WITH EXPOSED REINFORCEMENT: ADOPT SPALL REPAIR METHODOLOGY OUTLINE IN SECTION 2
- SURFACE DETERIORATION: ADOPT SURFACE REPAIR METHODOLOGY OUTLINE IN SECTION 3



SOUTHERN ABUTMENT AND WING WALL
SCALE 1:100



NORTHERN ABUTMENT AND WING WALL
SCALE 1:100

NOT FOR CONSTRUCTION

Rev	Description	Checked	Approved	Date
P01	90% FINAL DESIGN	AR	LR	07.06.24
Designer	C. NGUYEN	Design Check	A. RITCHIE	



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Project No.
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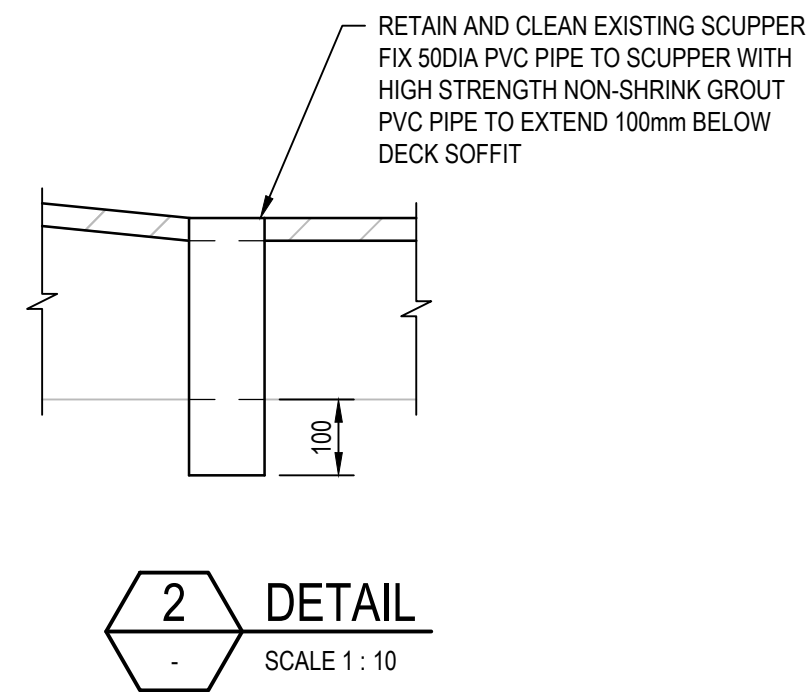
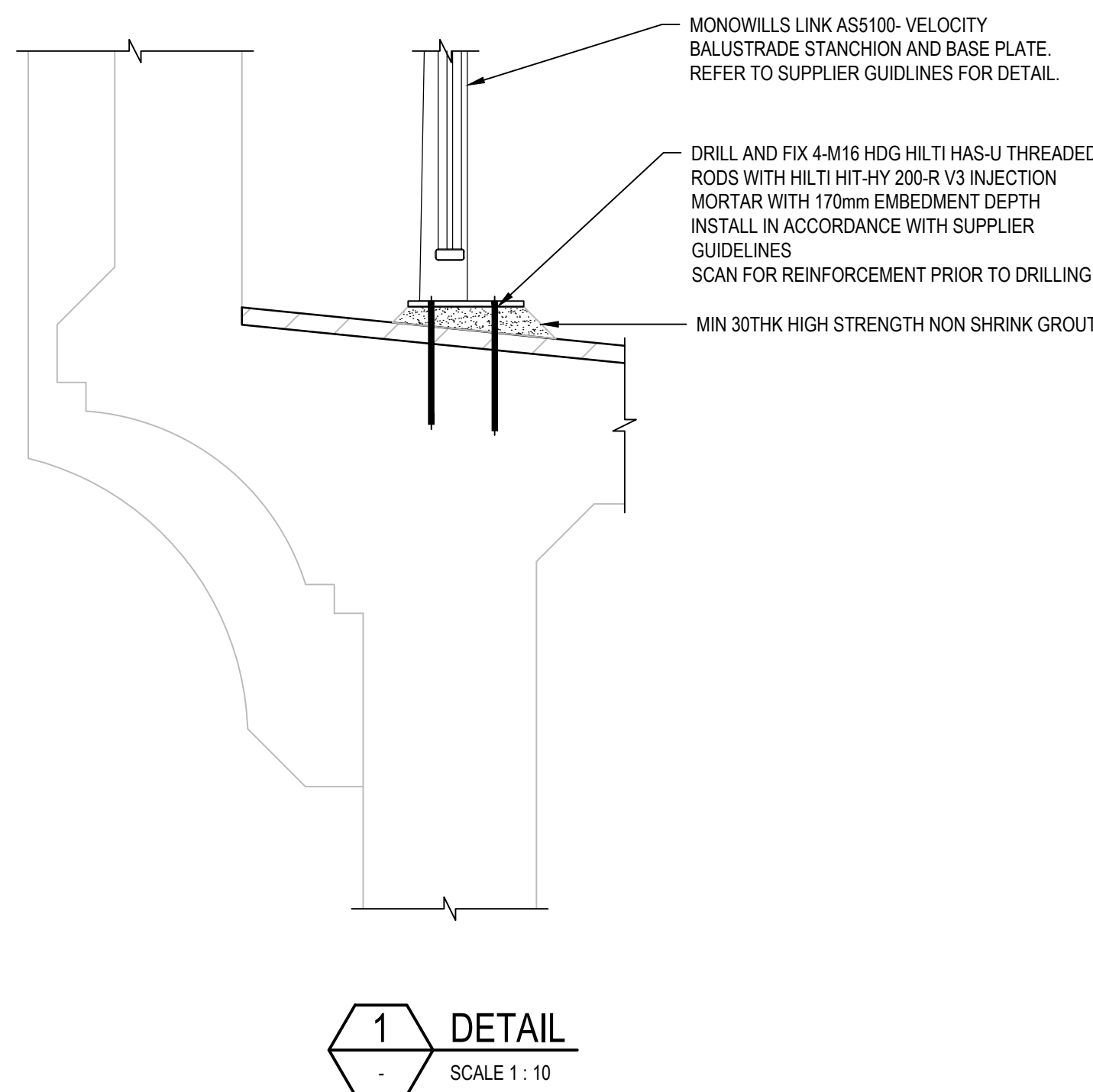
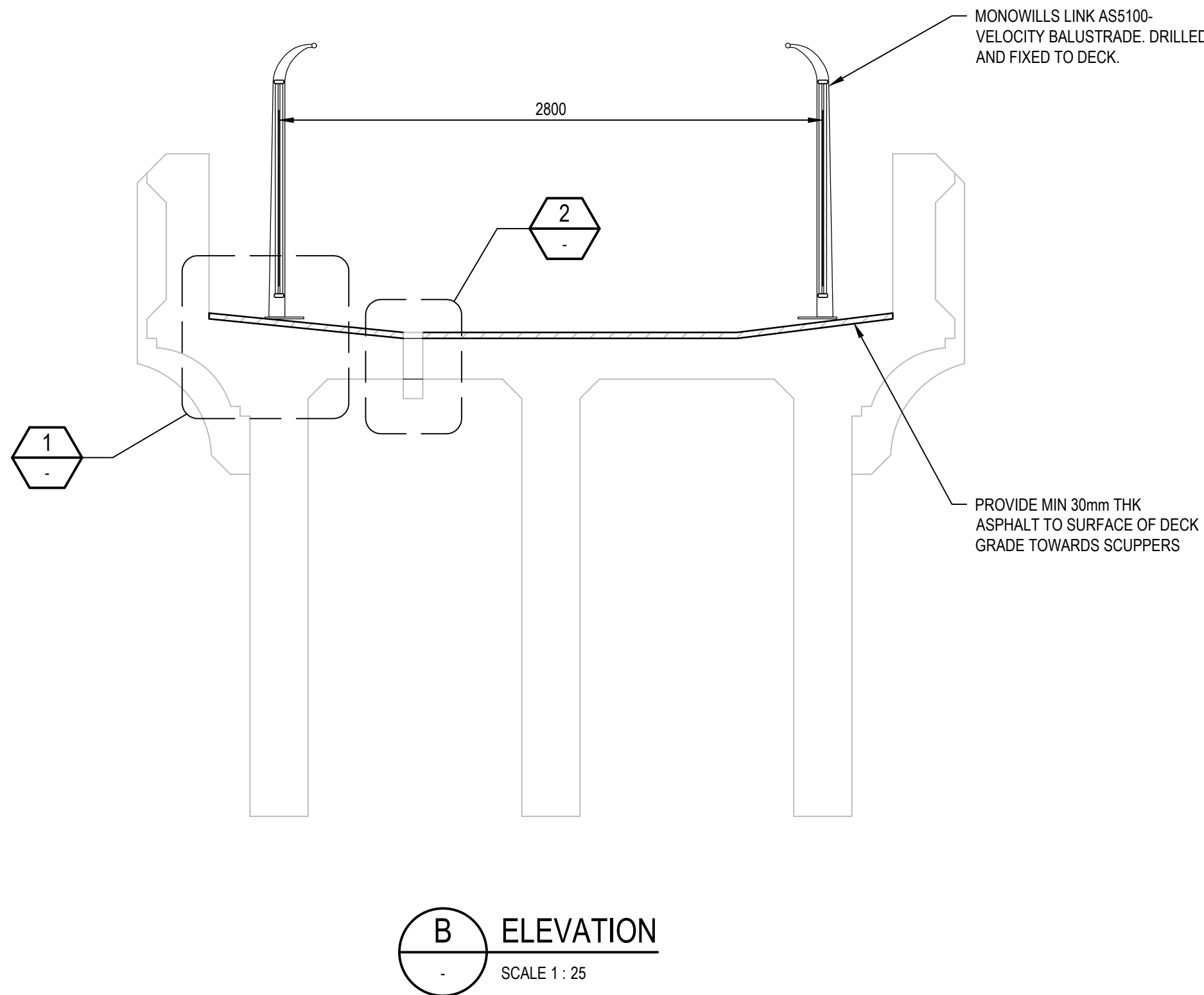
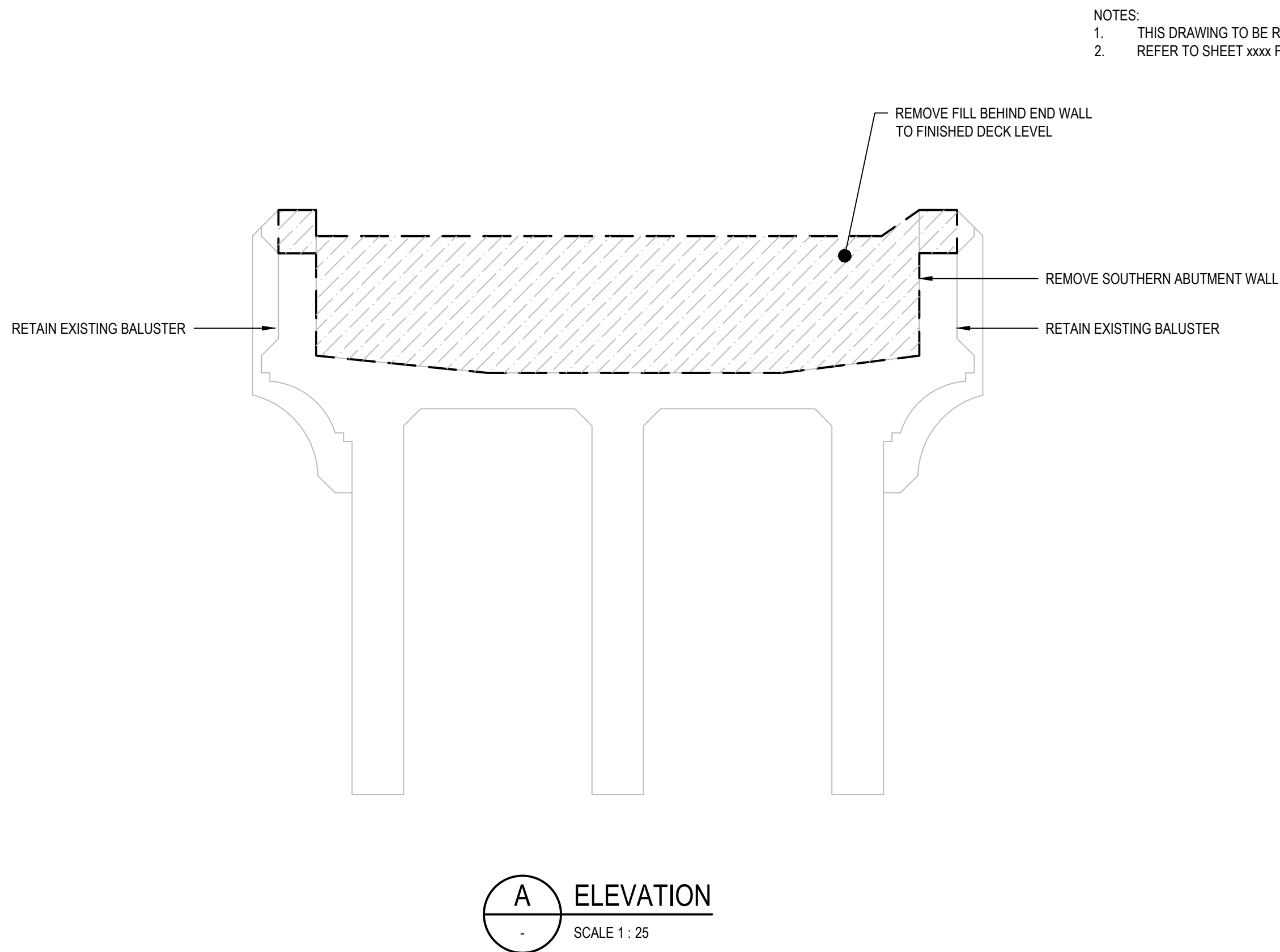
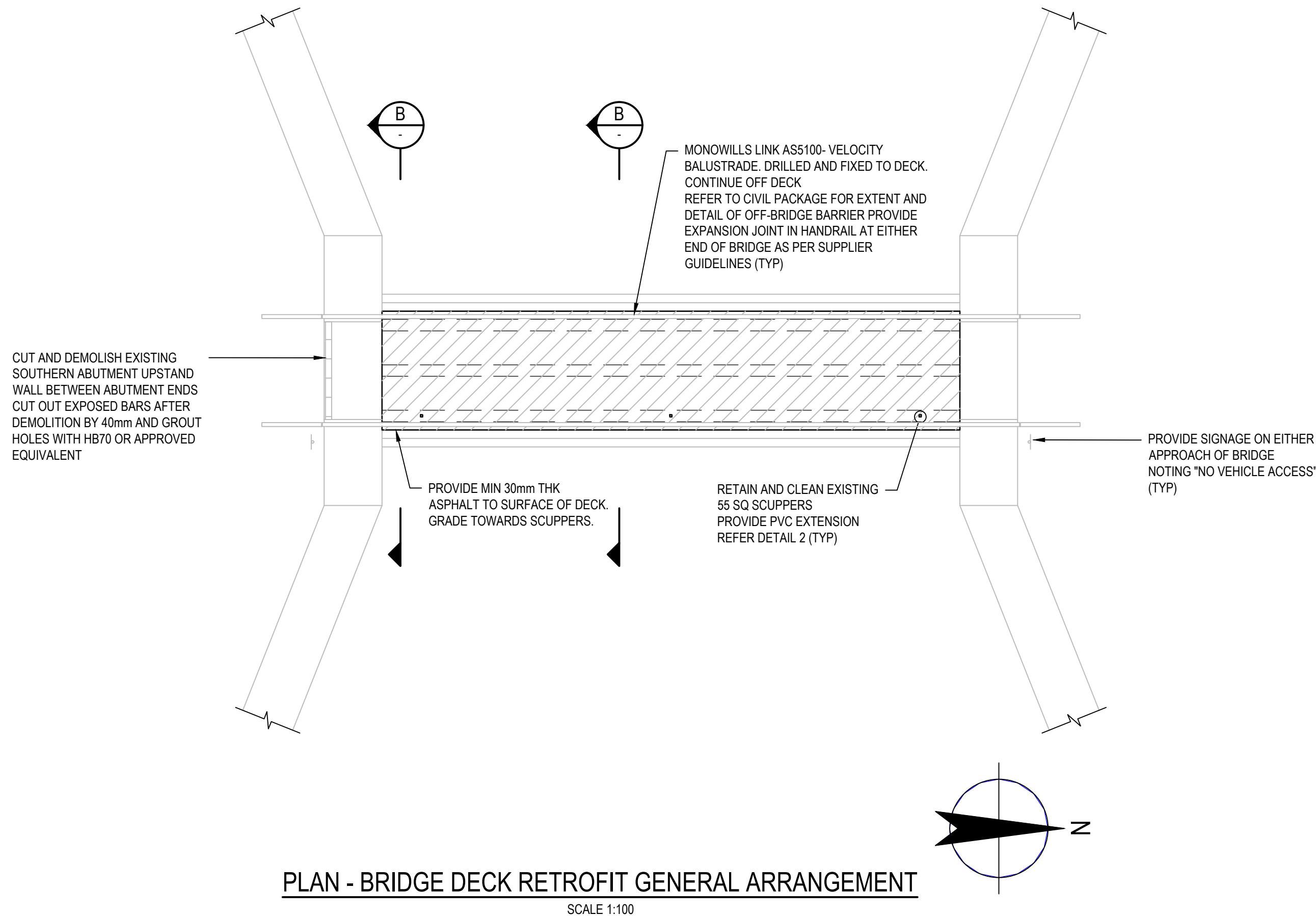
Client	ADELAIDE HILLS COUNCIL
Project	AMY GILLET BIKEWAY
Status	PRELIMINARY

Drawing Title
BURFORDS HILL ROAD BRIDGE -
REMEDIAL WORKS
PLAN AND ELEVATION

Drawing No.
12638597-GHD-00-00-DRG-ST-0040

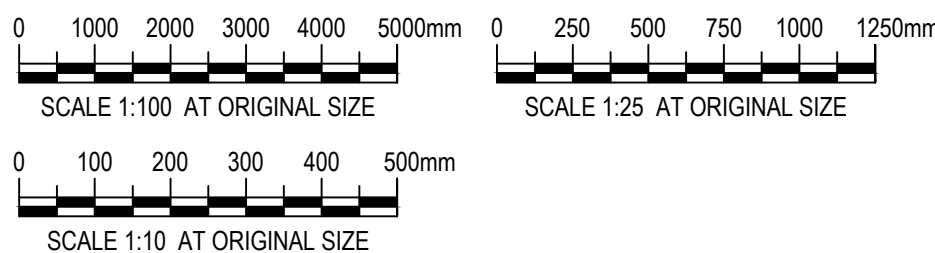
Size
A1

Rev
P01



NOT FOR CONSTRUCTION

Rev	Description	Checked	Approved	Date
P01	90% FINAL DESIGN	AR	LR	07.06.24
Author	A. PERNA	Drafting Check	L. PARRY	
Designer	C. NGUYEN	Design Check	A. RITCHIE	



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Project No.
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Client	ADELAIDE HILLS COUNCIL
Project	AMY GILLET BIKEWAY
Status	PRELIMINARY

Drawing Title	BURFORDS HILLS ROAD BRIDGE - DECK RETROFIT DETAILS
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12638597-GHD-00-00-DRG-ST-0040

Size
A1

Rev
P01